

On Simulation and Optimization of Freeway Network Operations

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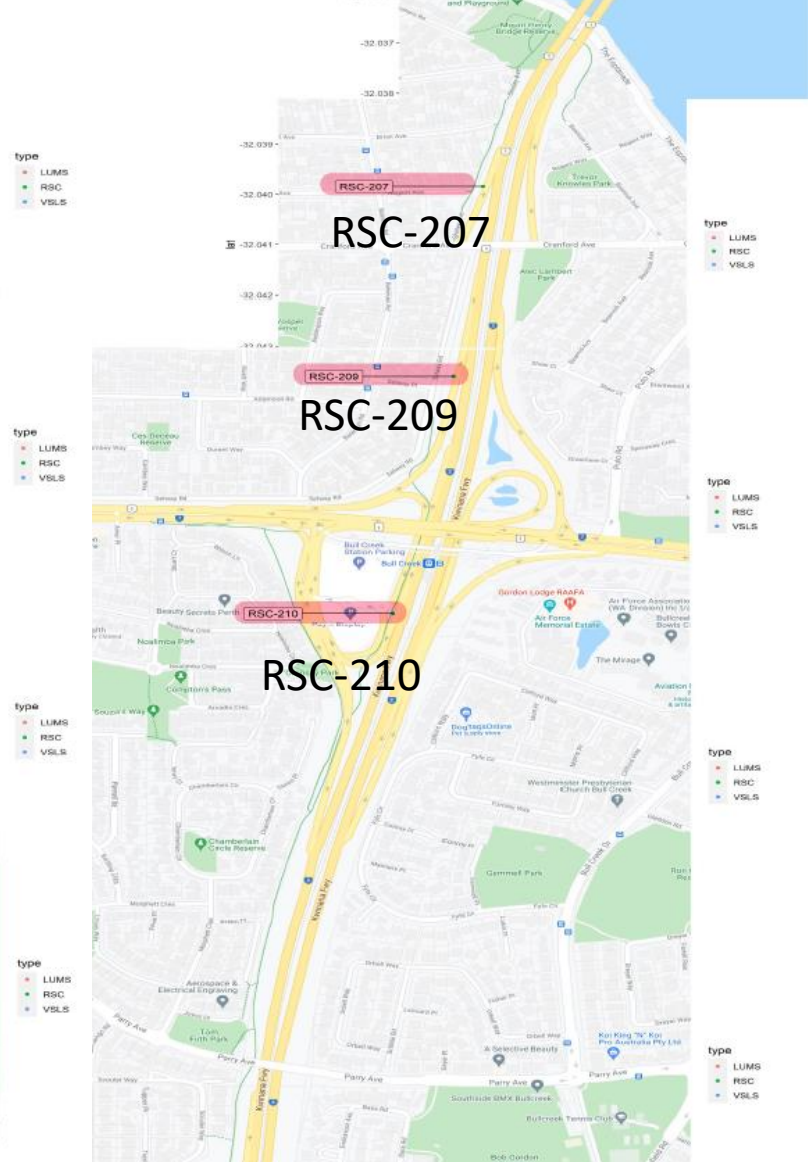
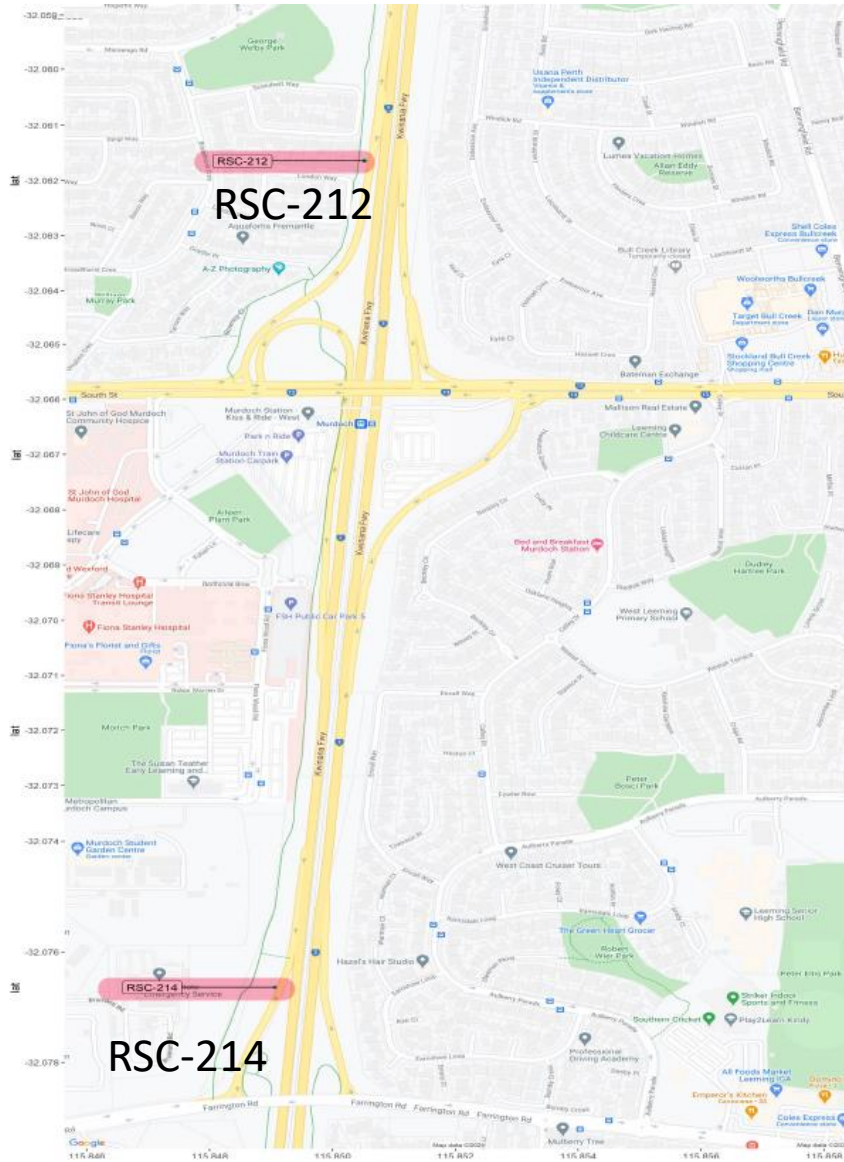


Progress from last PSG meeting

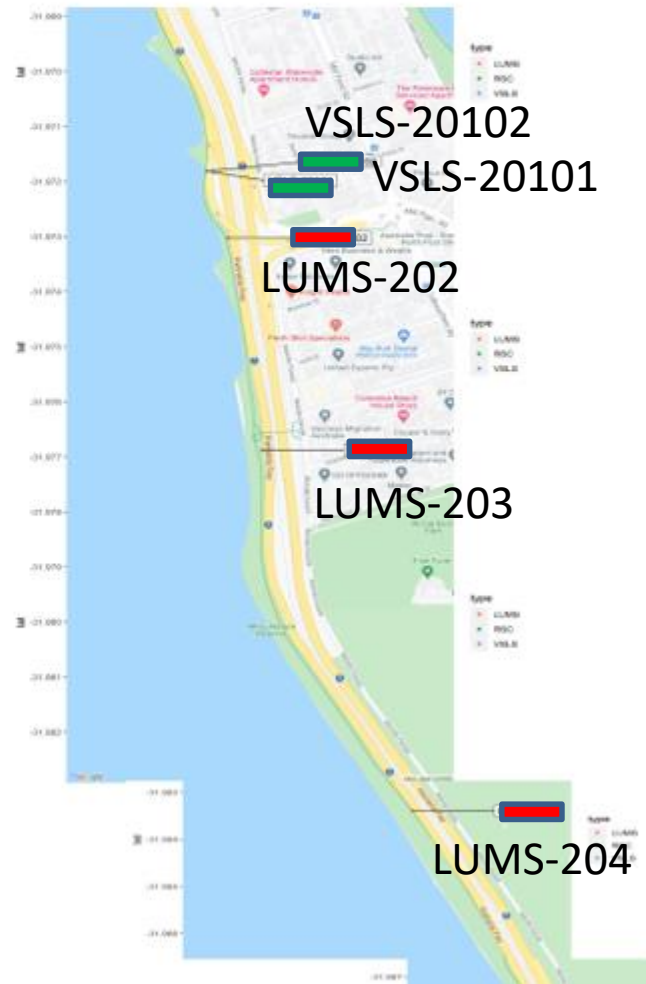
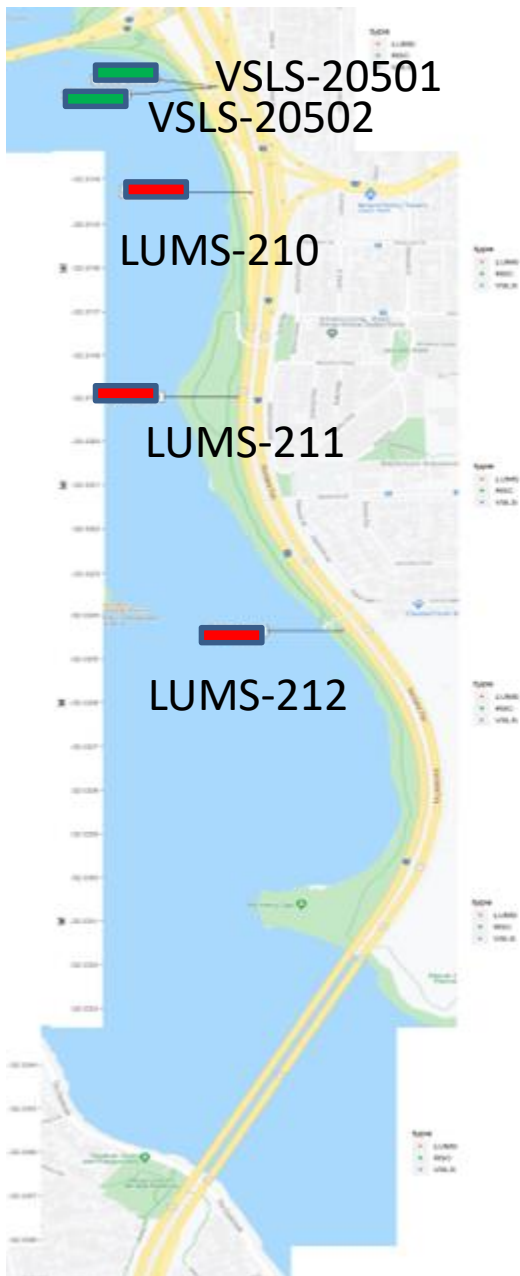
- Computer implementation of ramp metering and VSL on the Kwinana freeway traffic flow simulation model
- Further data analysis: traffic prediction by LSTM & BAM-LSTM methods
- Optimization of freeway traffic flow via ramp metering

1. Computer implementation of ramp metering and VSL on the Kwinana freeway traffic flow simulation model

Ramp Metering Series



Variable Speed Limit Signs



 VLS

 LUMS

RSC - I

75:5:10

60:5:25

60:5:25

60:5:25

75:5:10

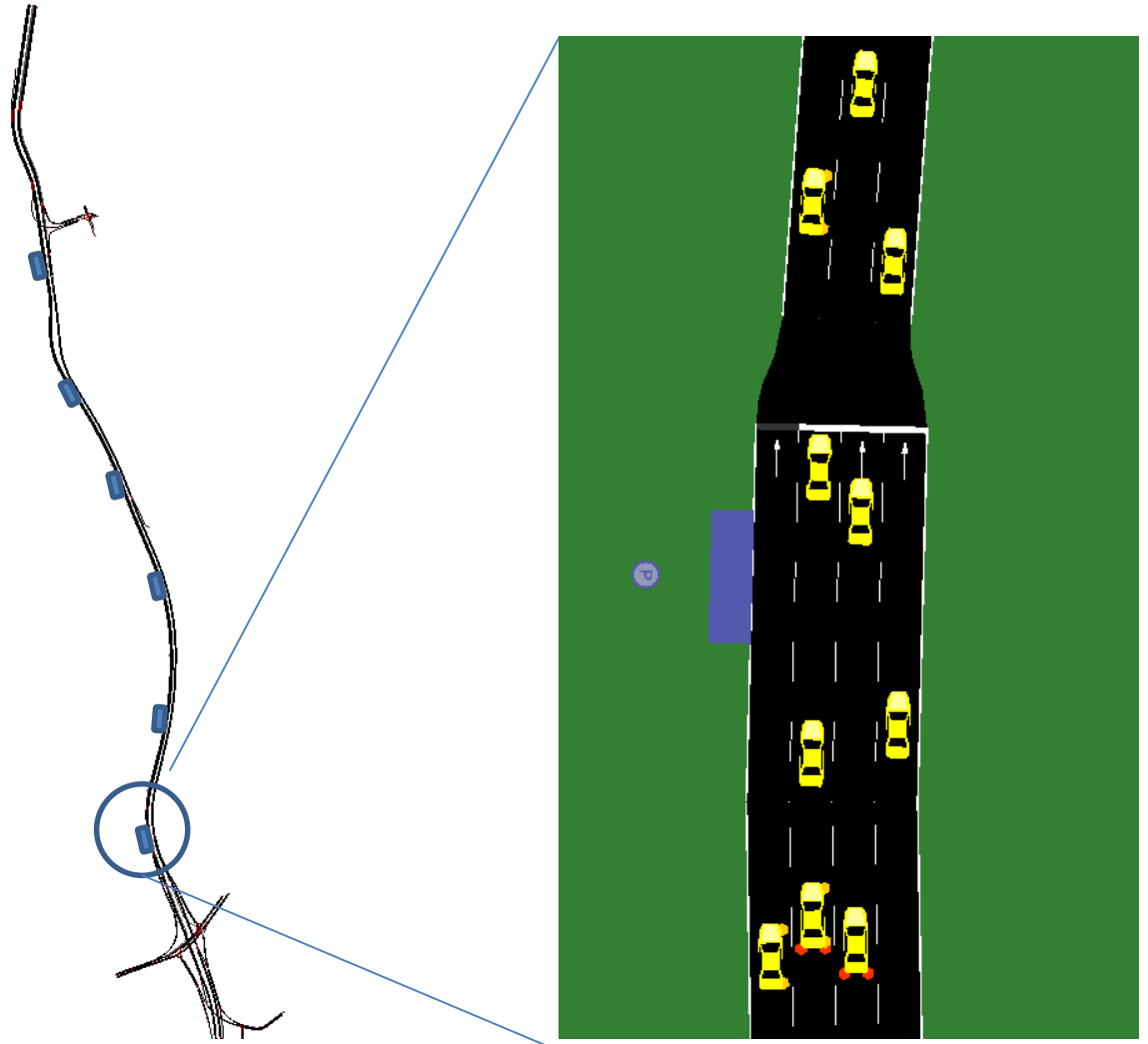
VSL

Test Schemes

1-3

Ramp Metering

6 Parking areas



Numerical Test Schemes

VSL Period: 6:00-10:00; 14:00-18:00

VSL Scheme	Ramp Metering (90 sec per cycle)	LUMS Speed	VSL Speed
1	80 G 5 Y 5 R	90	70
2	80 G 5 Y 5 R	100	70
3	RSC - I	90	70
4	RSC - I	100	70

VSL Application Design



LUMS and VSLS

- R201 202 203 204 205 206 207 208 R203 209 R205 210 211 212 ALL

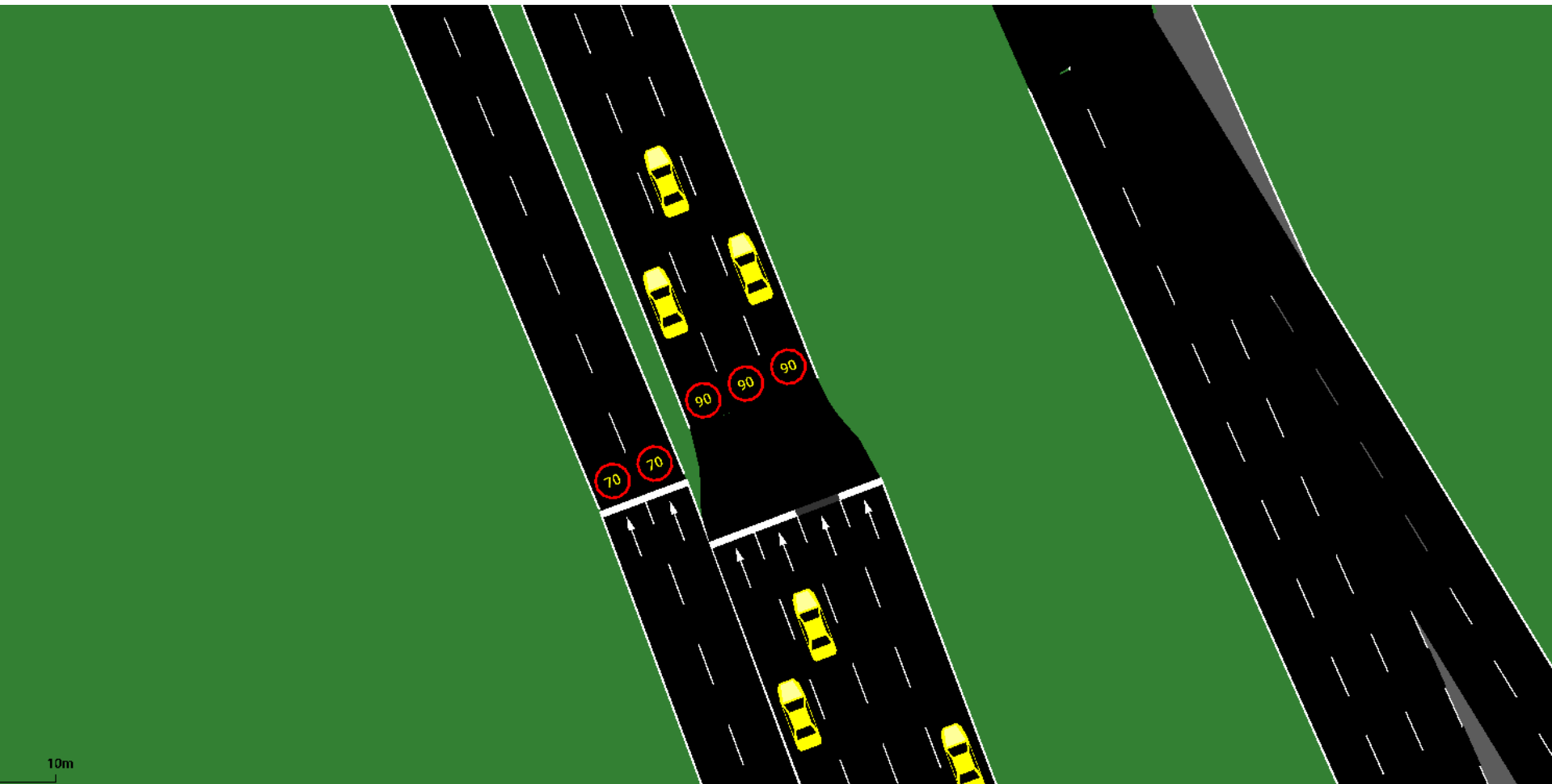
TIME PERIOD

- 00:00-23:59 6:00-18:00 6:00-10:00; 14:00-18:00

MAX Speed (km/h)

- 70 80 90 100

Push Button

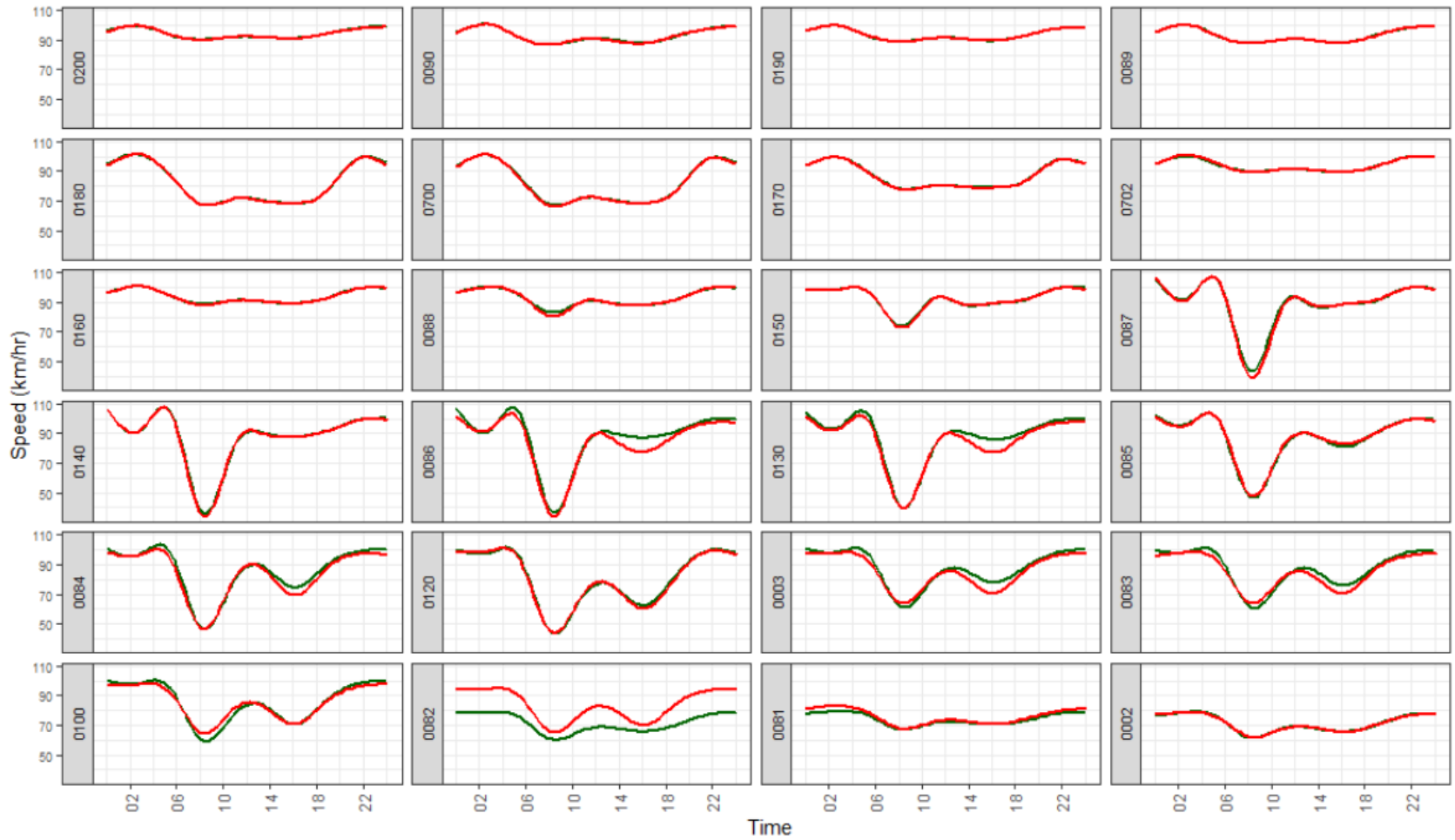


VLS (70 km/h) and LUMS (90 km/h)

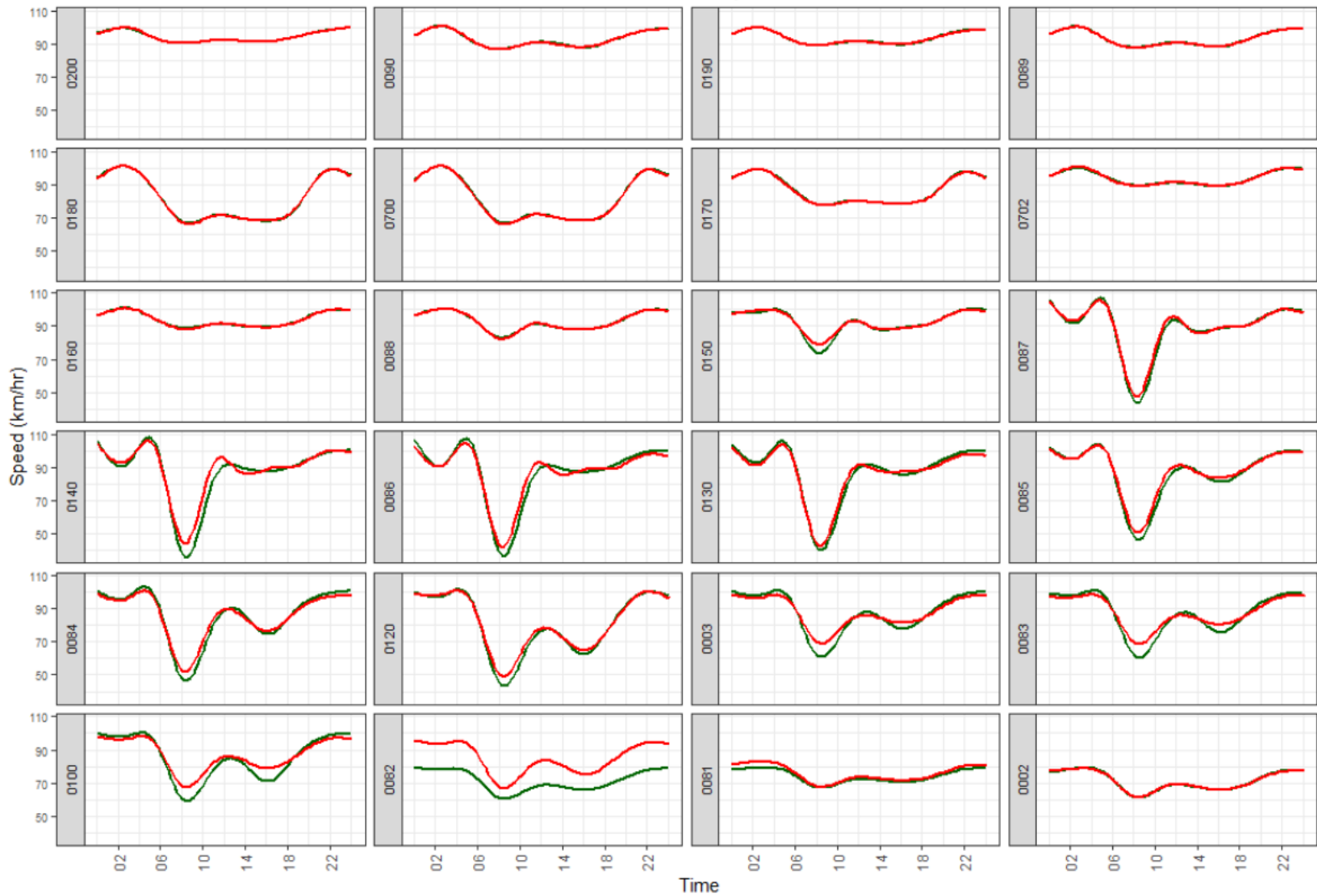


Microsimulation

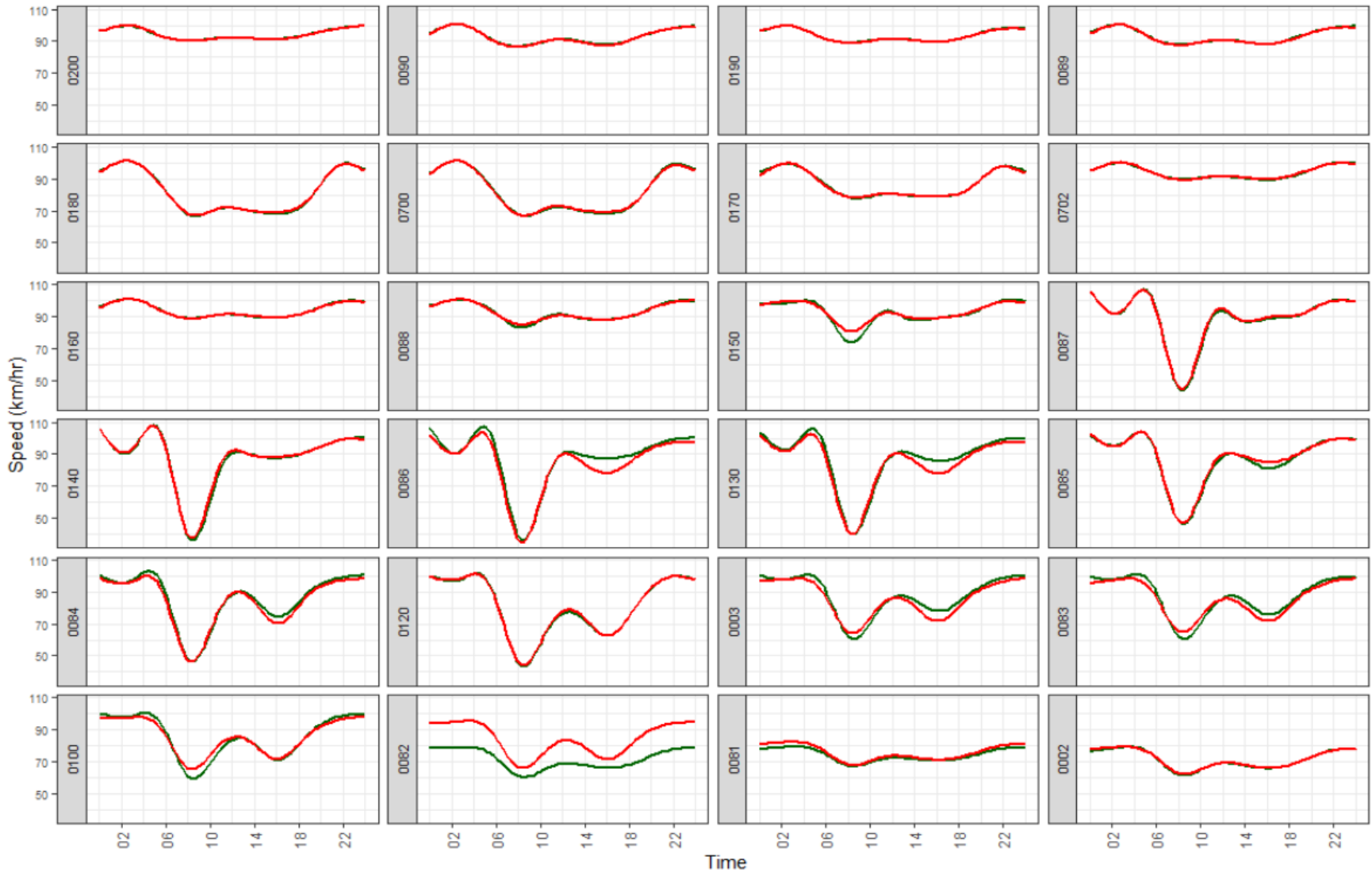
LUMS 90 & VSLS 70



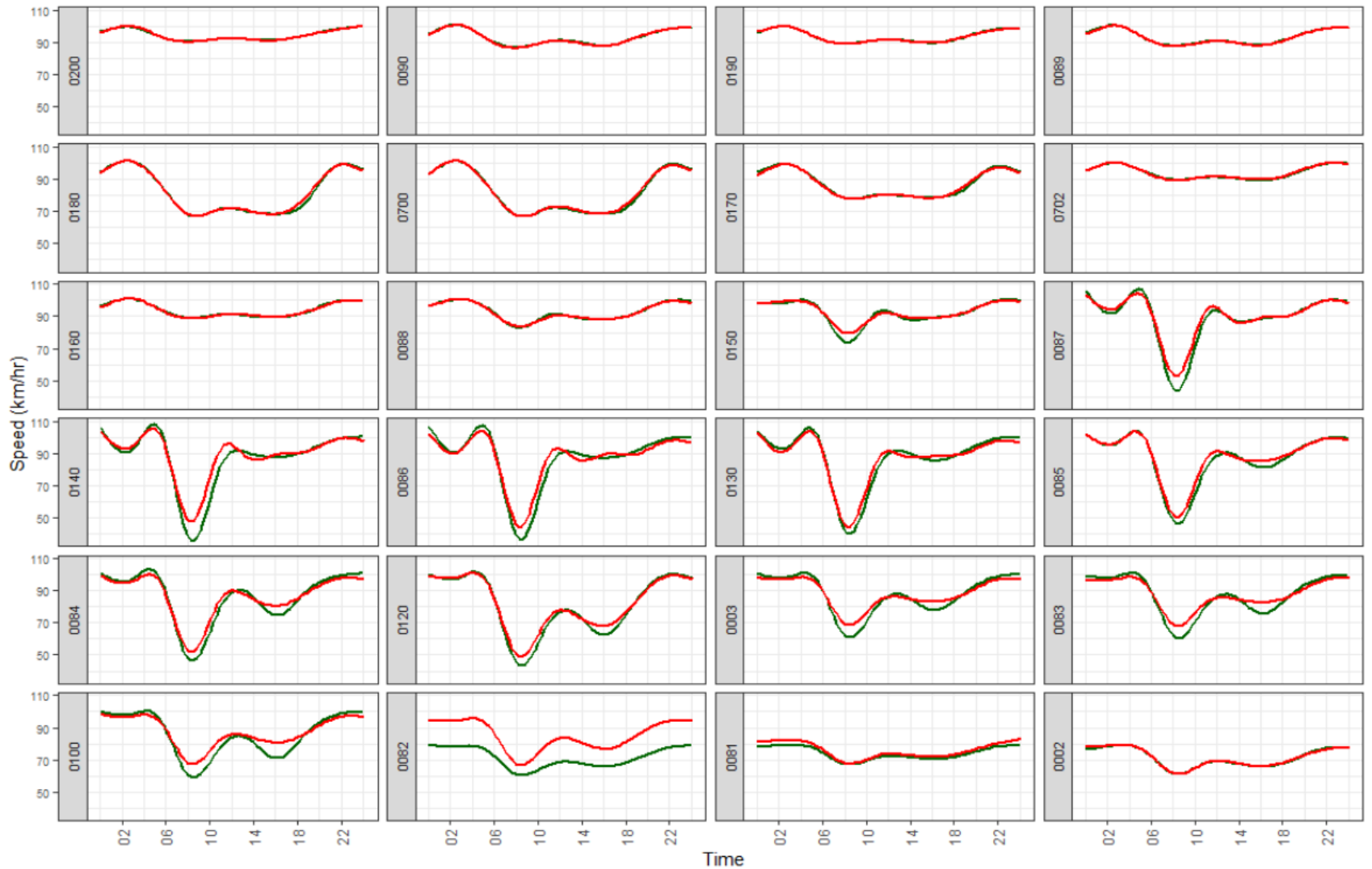
LUMS 100 & VSLS 70

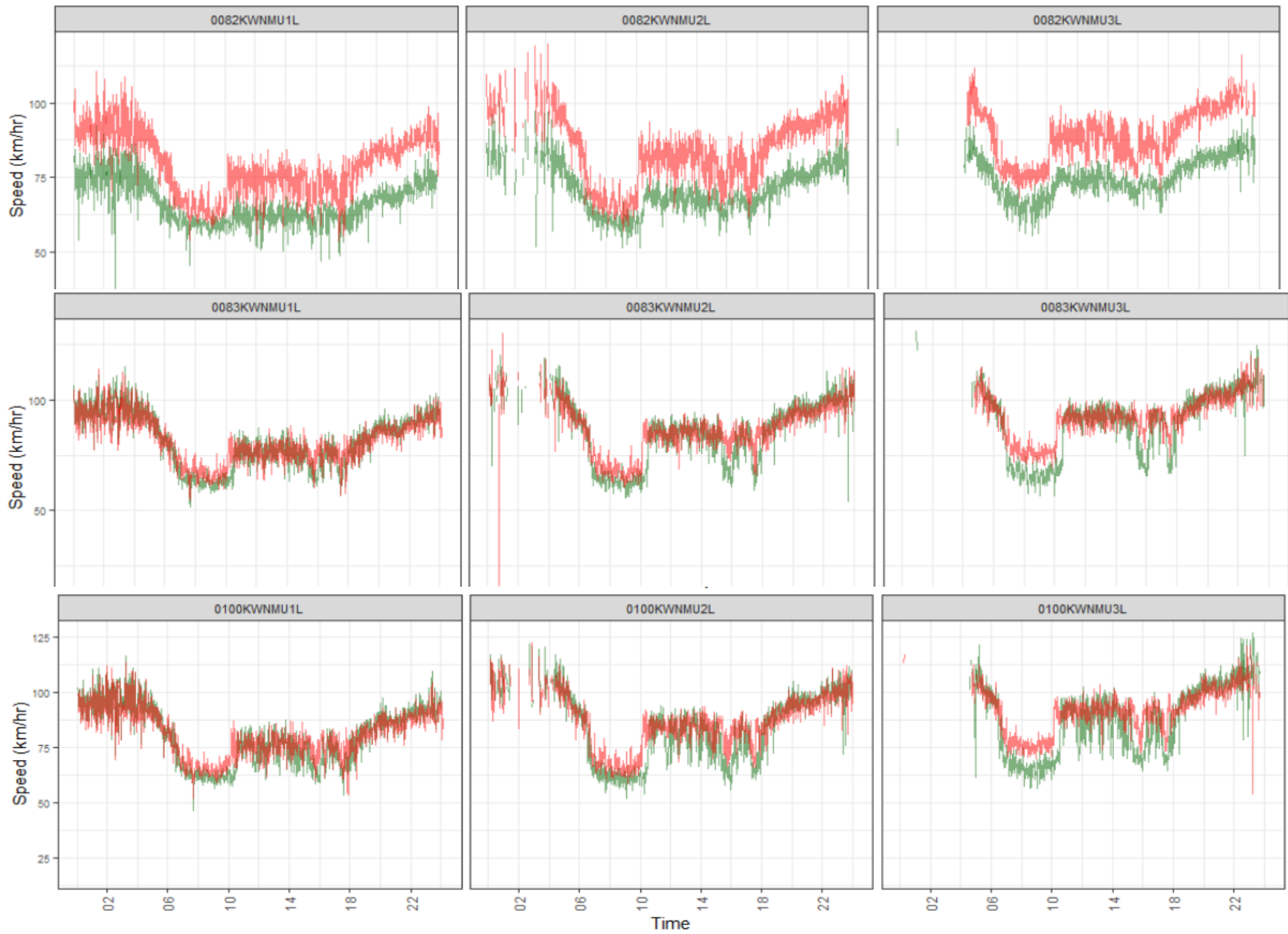


LUMS 90 & VSLS 70 & RSC1 (from 6 AM)



LUMS 100 & VSLS 70 && RSC (from 6 AM)



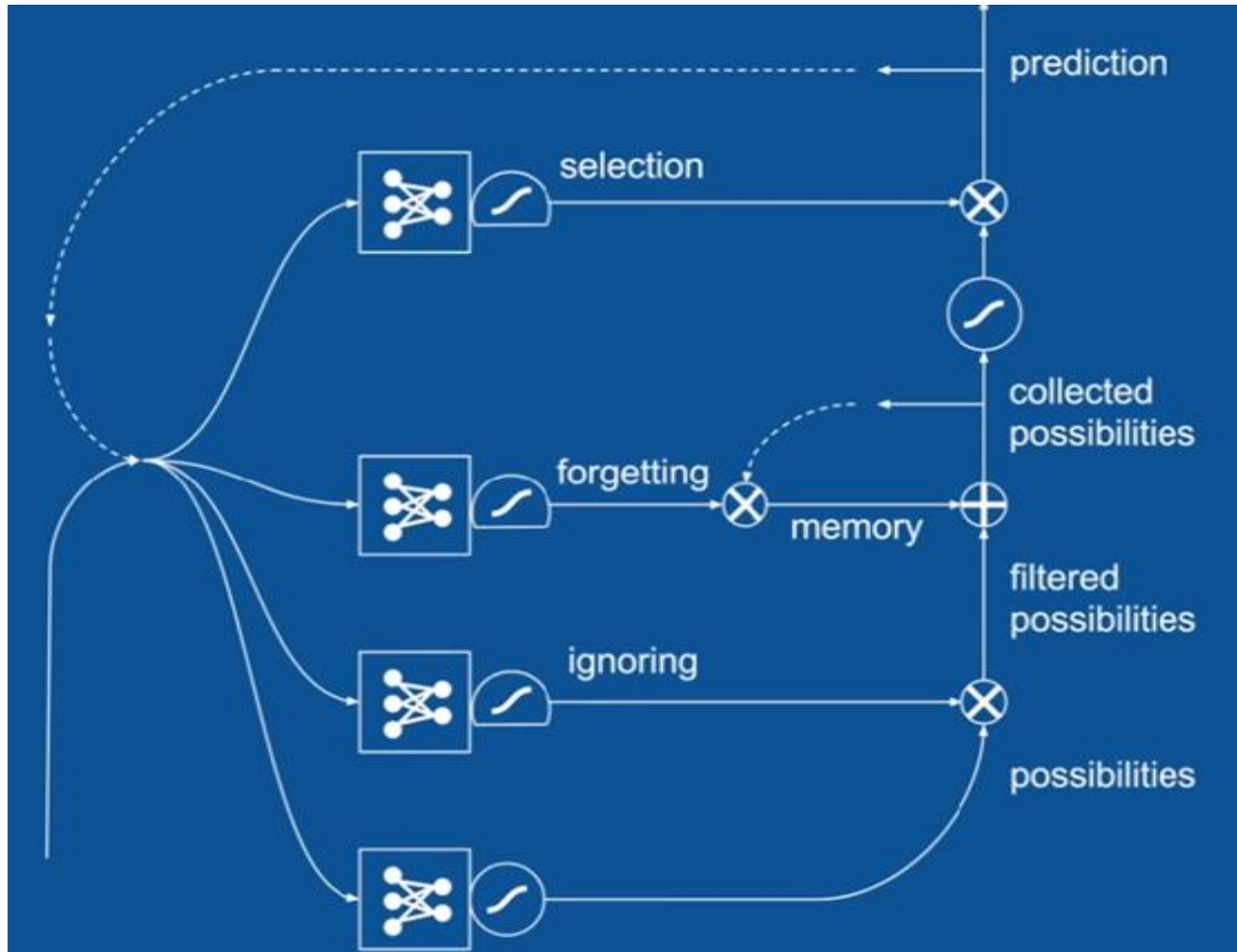


Statistics (Average)

Scheme	Duration (s)	Waiting Time(s)	Time Loss(s)	Depart Delay(s)
n/a	391.17	8.3	120.73	97.74
1	391.97	8.21	117.36	97.34
2	378.56	6.81	110.13	95.53
3	385.83	9.04	111.58	93.90
4	373.15	8.64	105.16	92.09

2. Further data analysis: traffic prediction by LSTM & BAM-LSTM methods

LSTM Cell



Traditional LSTM with forget gates*

Initial values $c_0 = 0$ and $h_0 = 0$.

The operator 'o' denotes the Hadamard product

$$f_t = \sigma_g(W_f x_t + U_f h_{t-1} + b_f)$$

$$i_t = \sigma_g(W_i x_t + U_i h_{t-1} + b_i)$$

$$o_t = \sigma_g(W_o x_t + U_o h_{t-1} + b_o)$$

$$h_t = o_t \circ \sigma_h(c_t)$$

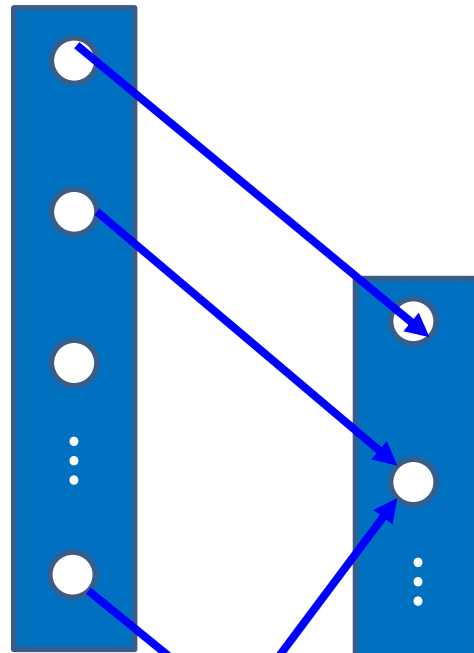
$$c_t = f_t \circ c_{t-1} + i_t \circ \sigma_c(W_c x_t + U_c h_{t-1} + b_c)$$

where σ_g : a sigmoid function

σ_c : a hyperbolic function

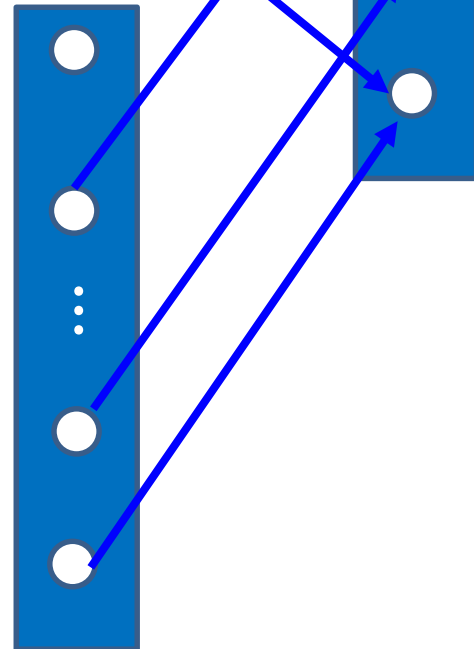
σ_h : a hyperbolic tangent function

Prediction for
yesterday

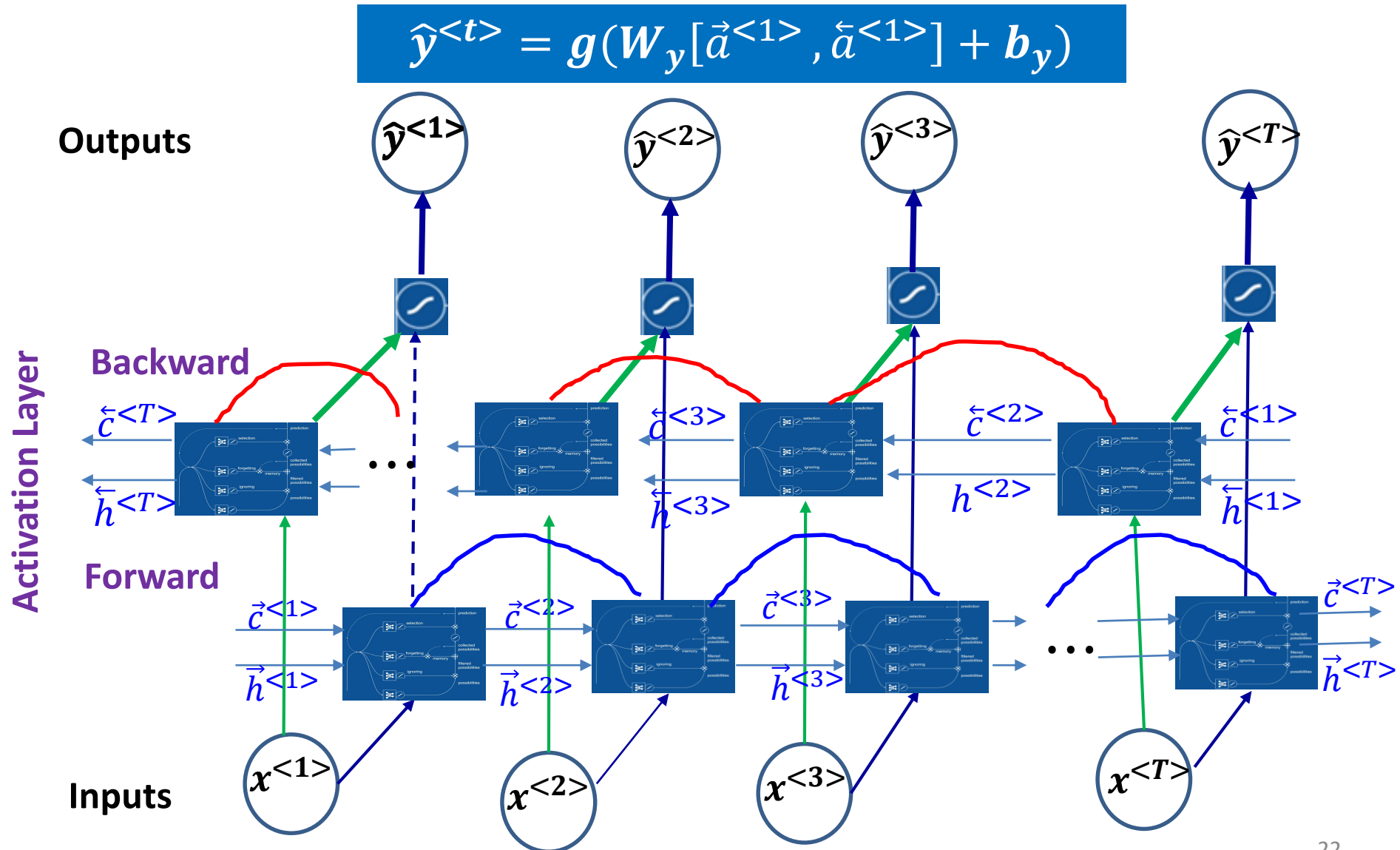


Prediction for
today

Traffic flow
rate for
yesterday



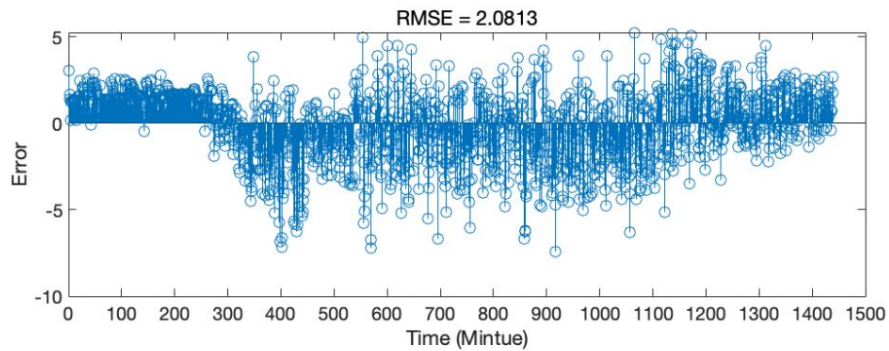
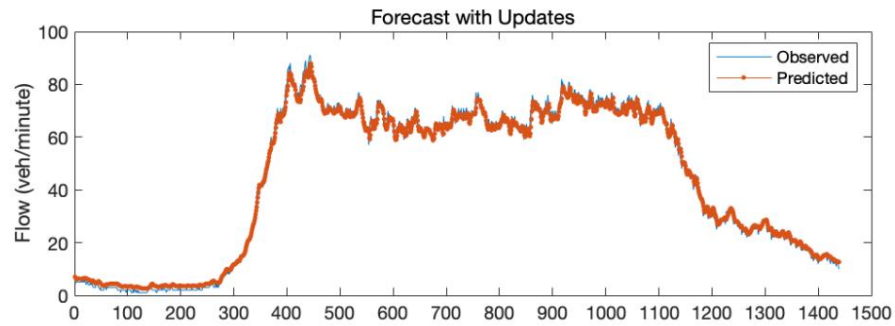
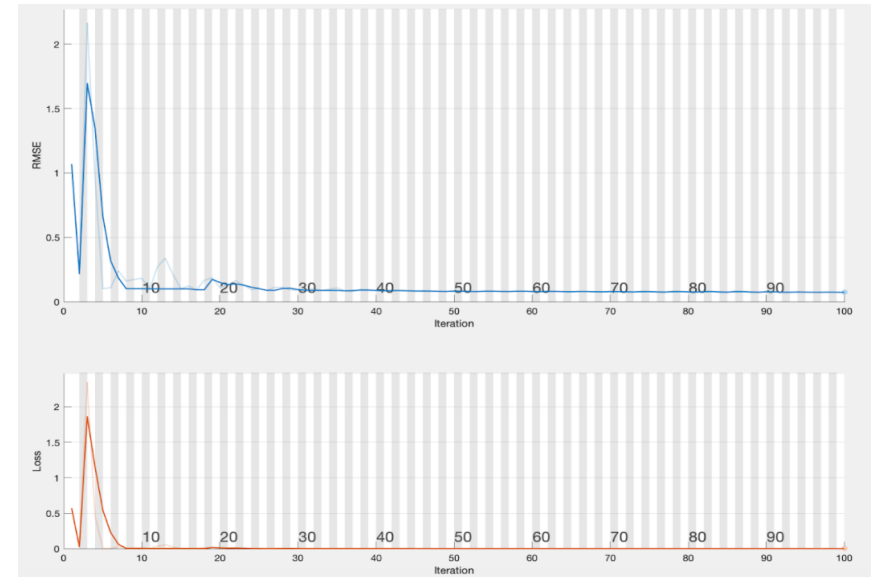
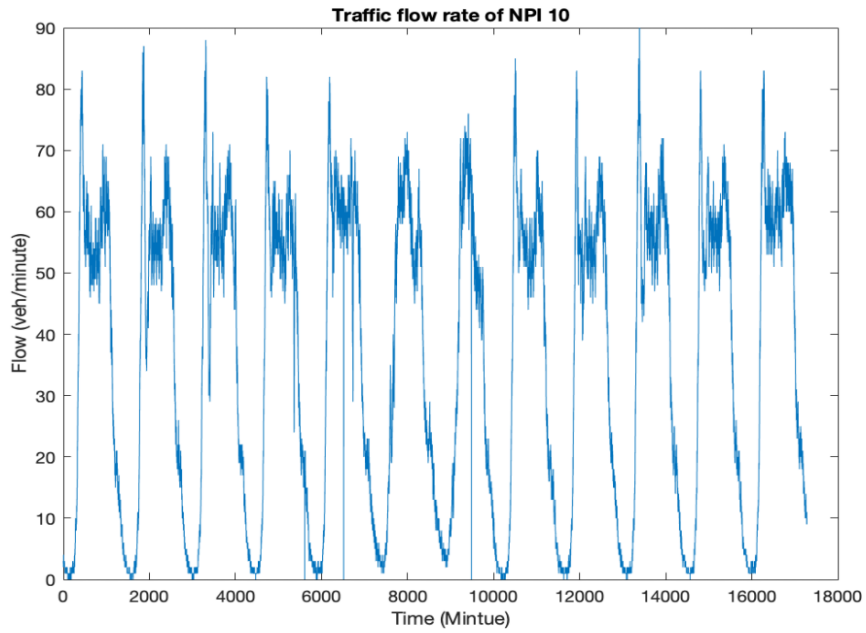
Bidirectional Recurrent Neural Network

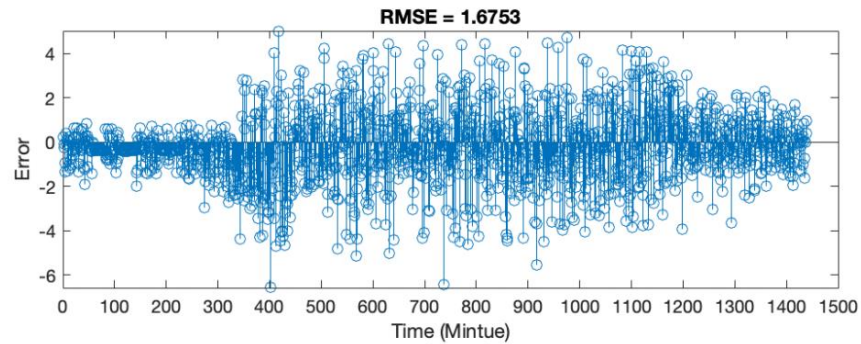
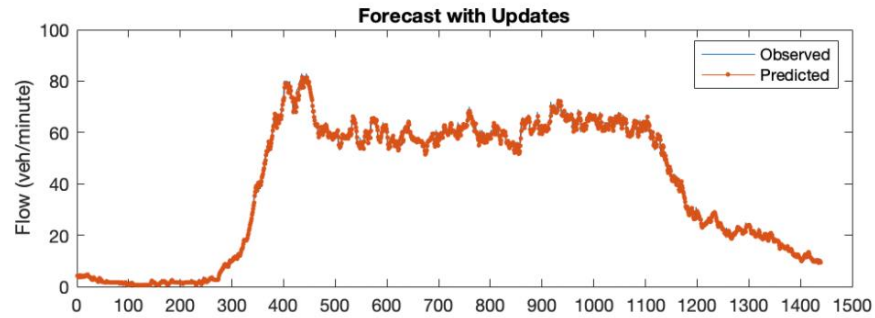
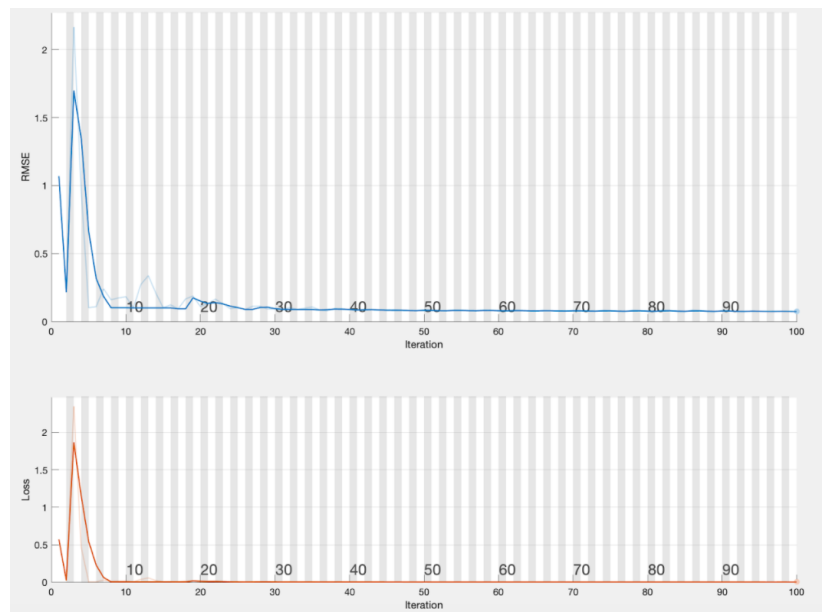
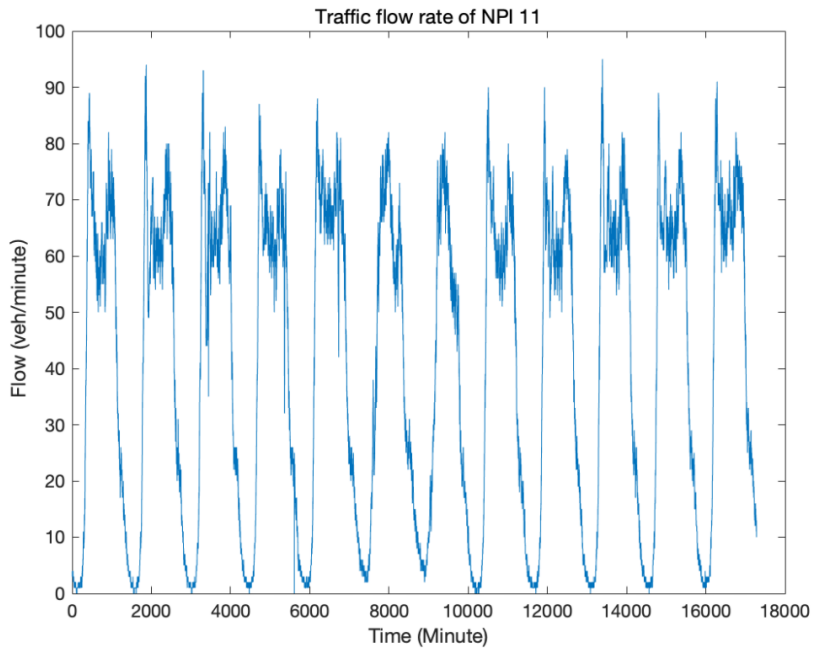


Hyperparameters for BRNN

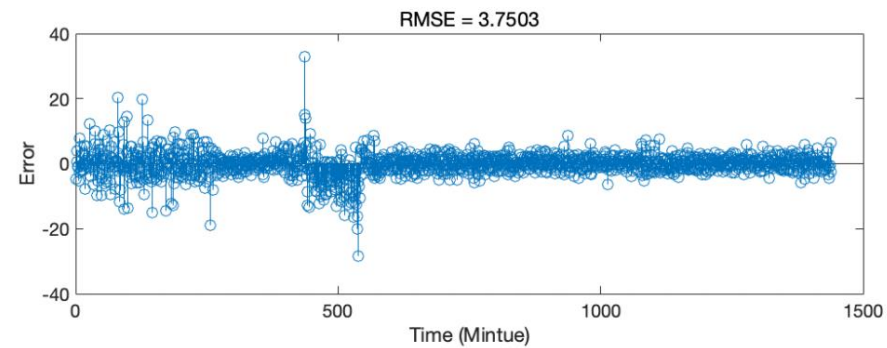
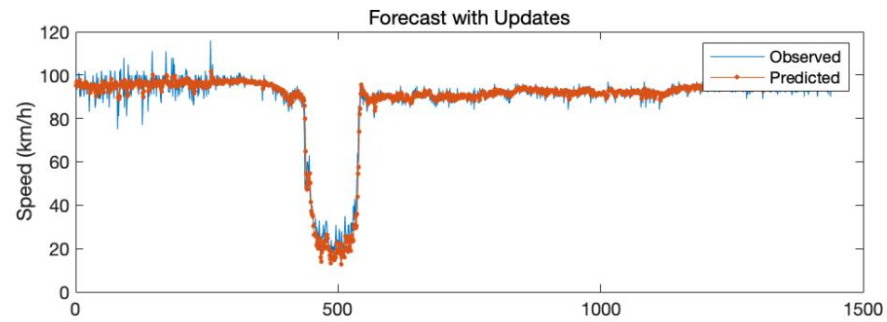
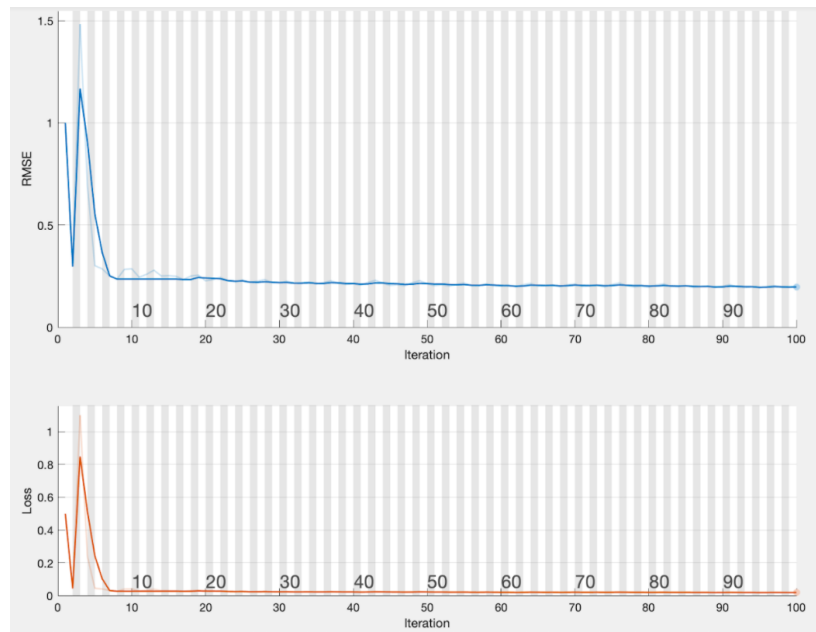
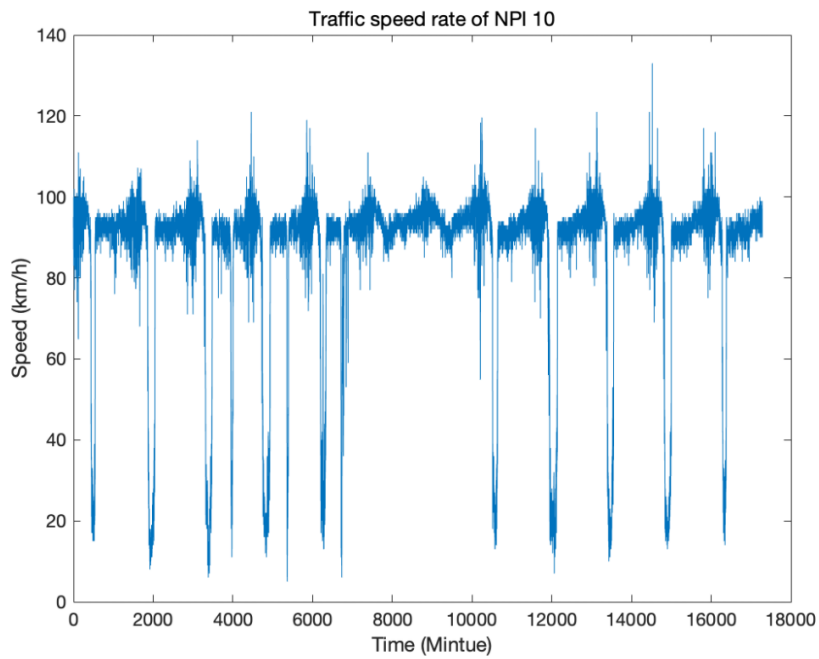
- Input size = 144000
- Sequence length = 1440
- Number of layers = 2
- Hidden size = 50
- Number of classes = 2
- Learning rate = 0.005
- Batch size = 120
- Number of epochs = 100

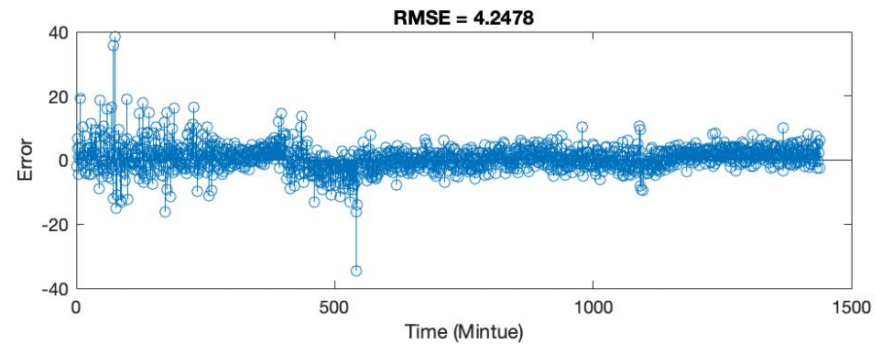
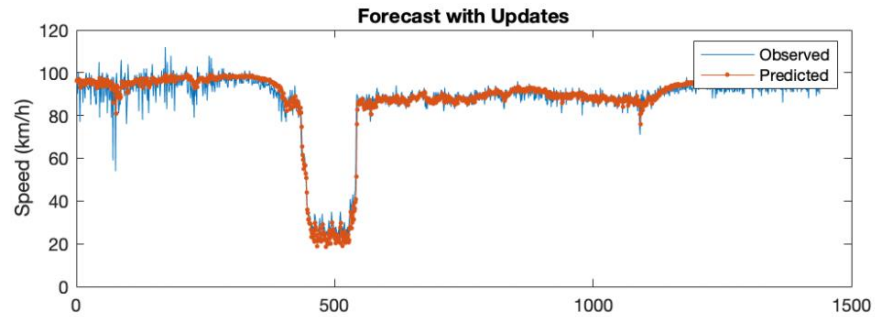
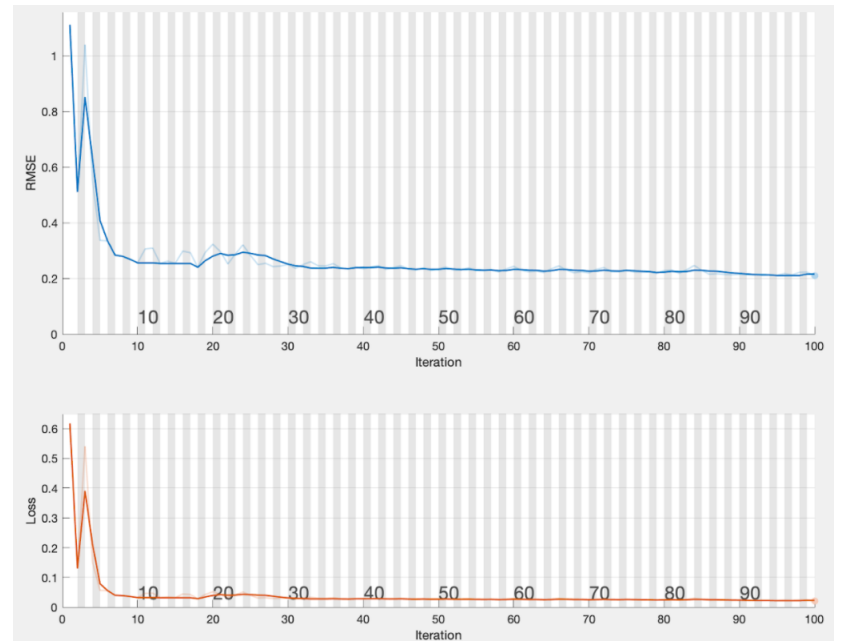
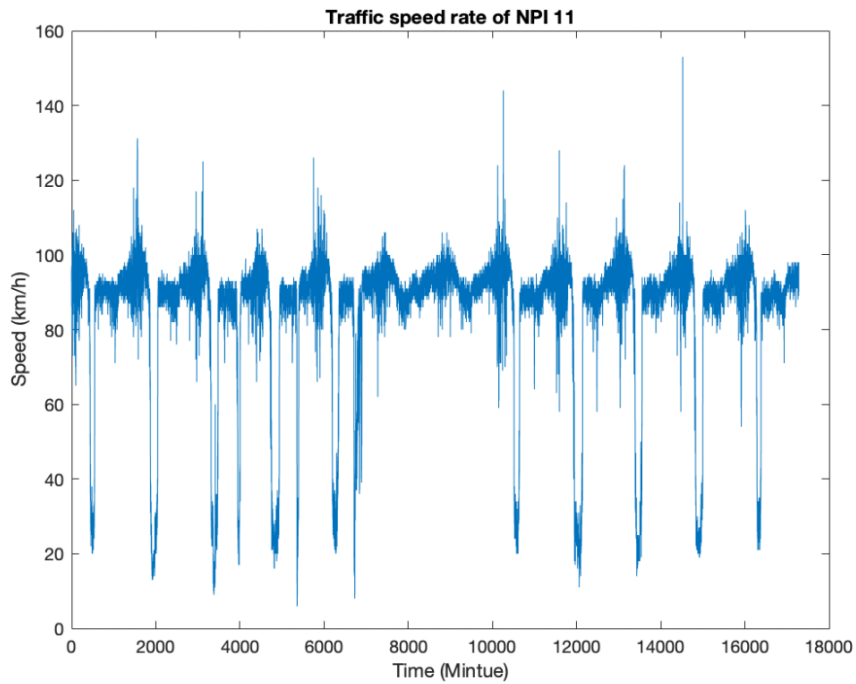
Traffic Flow Rate
Veh/minute



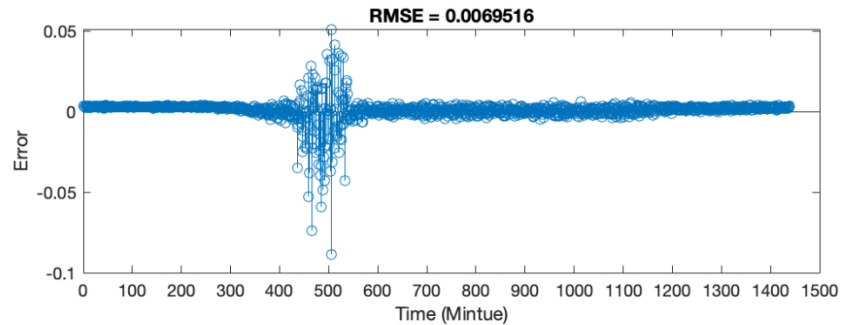
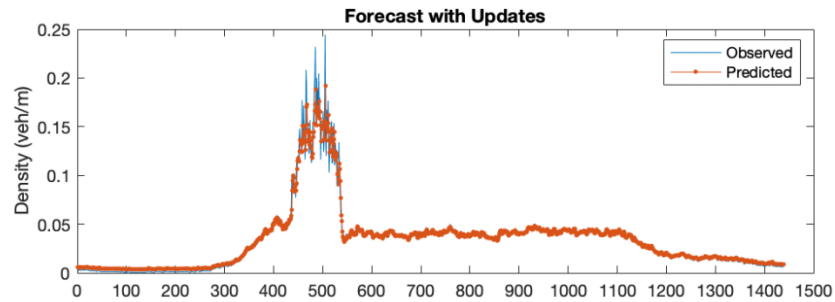
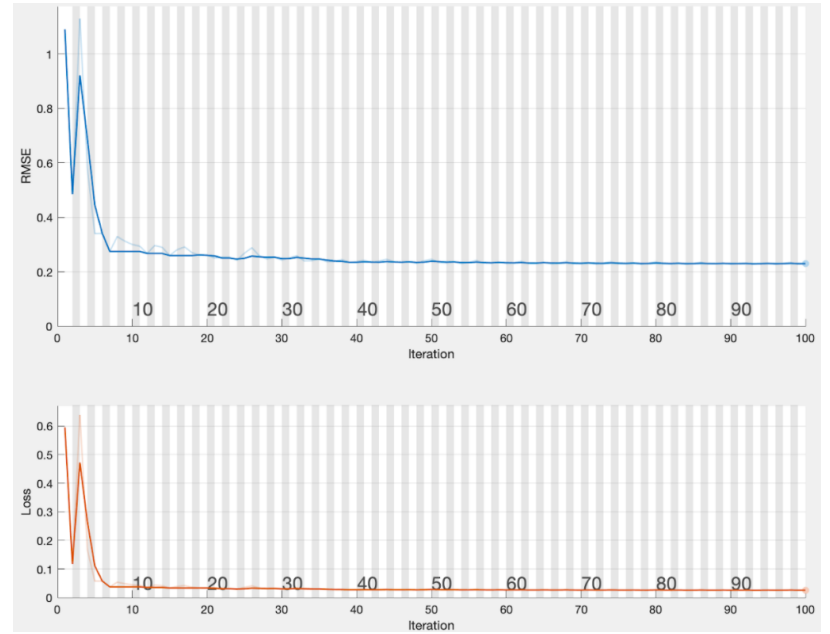
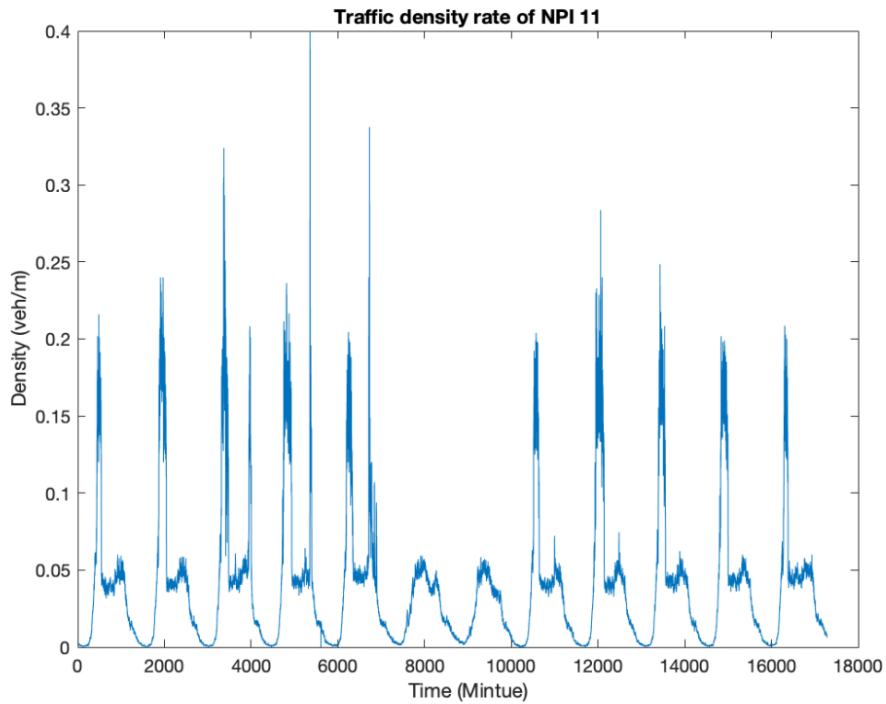


Traffic Speed
km/hr





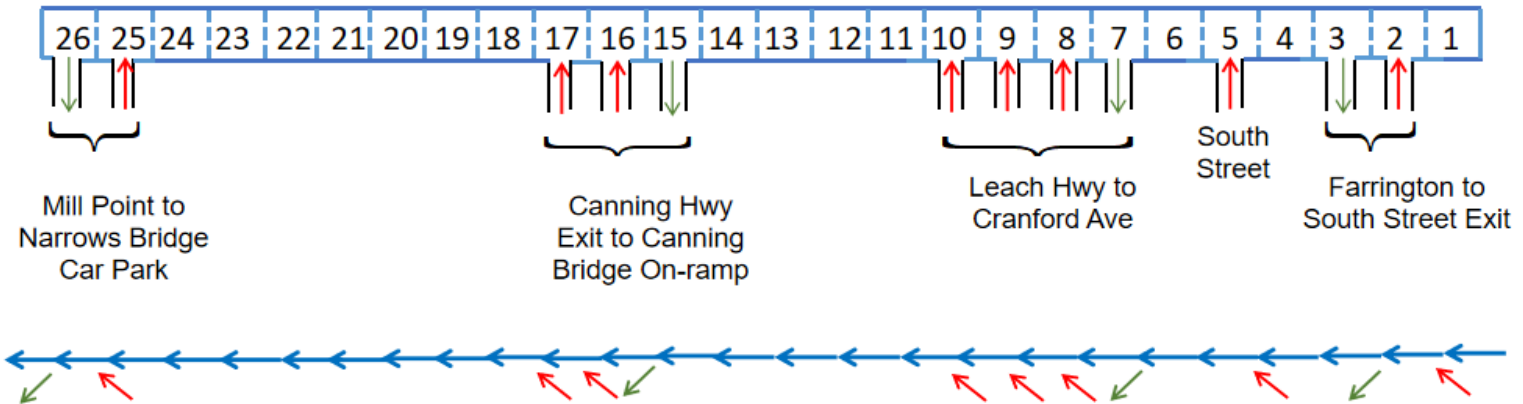
Traffic Density
Veh/meter



3 Optimization of freeway traffic flow via ramp metering

PhD Student: C. Gu; Supervisors: YH Wu & B Wiwatanapataphee

13 km
26 cells
8 on-ramps
4 off-ramps



Optimization Model

Objective function:
$$D_t = \sum_{l=t}^{t+N_p-1} \left[\sum_{i=1}^I (\rho_{i,l} \Delta x_i \Delta t - \frac{f_{i,l} \Delta x_i \Delta t}{v_i}) + \sum_{j=1}^J q_{j,l} \Delta t \right].$$

s.t.
$$\rho_{i,t+1}^\varepsilon = \rho_{i,t}^\varepsilon + \frac{\Delta t}{\Delta x_i} \times (f_{i-1,t}^\varepsilon(\rho_{i-1,t}^\varepsilon) - f_{i,t}^\varepsilon(\rho_{i,t}^\varepsilon) + r_{i,t} - s_{i,t}), \forall i, t,$$

$$q_{j,t+1} = q_{j,t} + (d_{j,t} - r_{j,t}) \Delta t, \forall j, t,$$

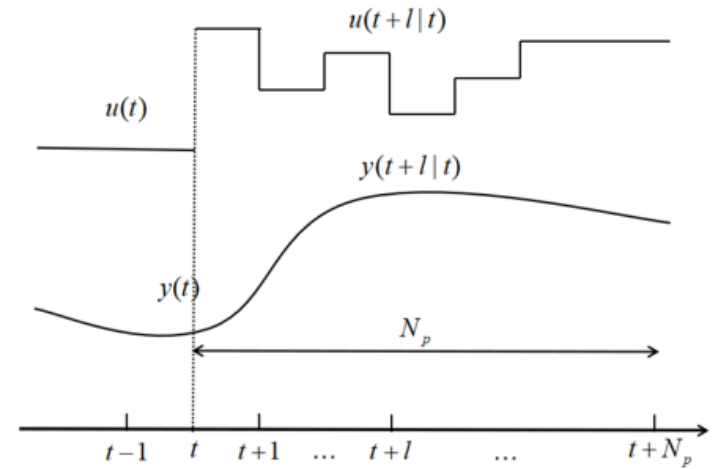
$$0 \leq \rho_{i,t}^\varepsilon \leq \rho_{max,i}, \forall i, t,$$

$$0 \leq q_{j,t} \leq q_{max}, \forall j, t.$$

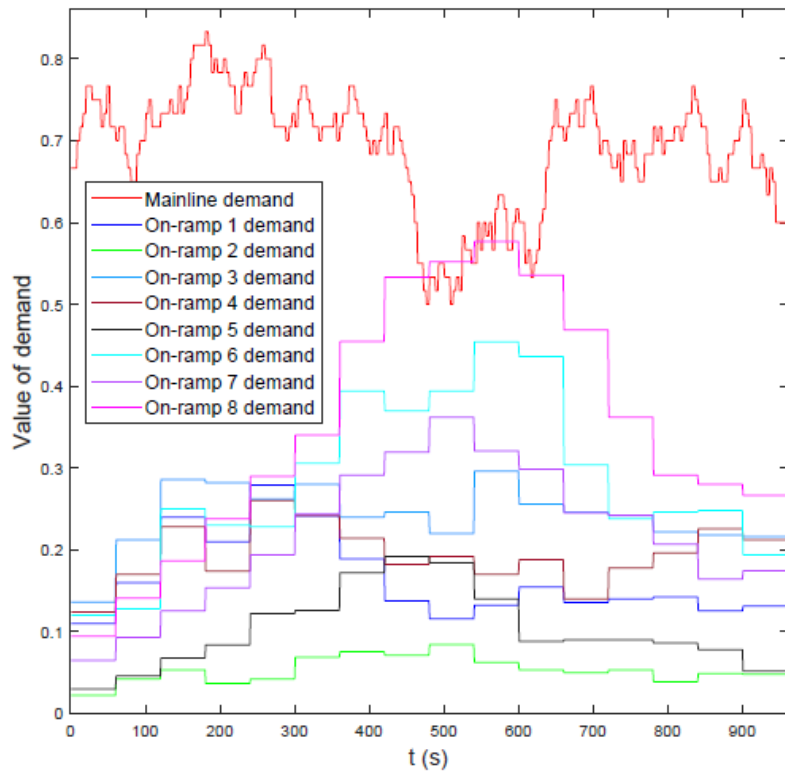
$$0 \leq r_{j,t} \leq r_{max,j}, \forall j, t.$$

$$f_{i,t} = \min\{v_i \rho_{i,t}, C_i, C_{i+1}, w_{i+1}(\rho_{max,i+1} - \rho_{i+1,t}), \forall i, t,$$

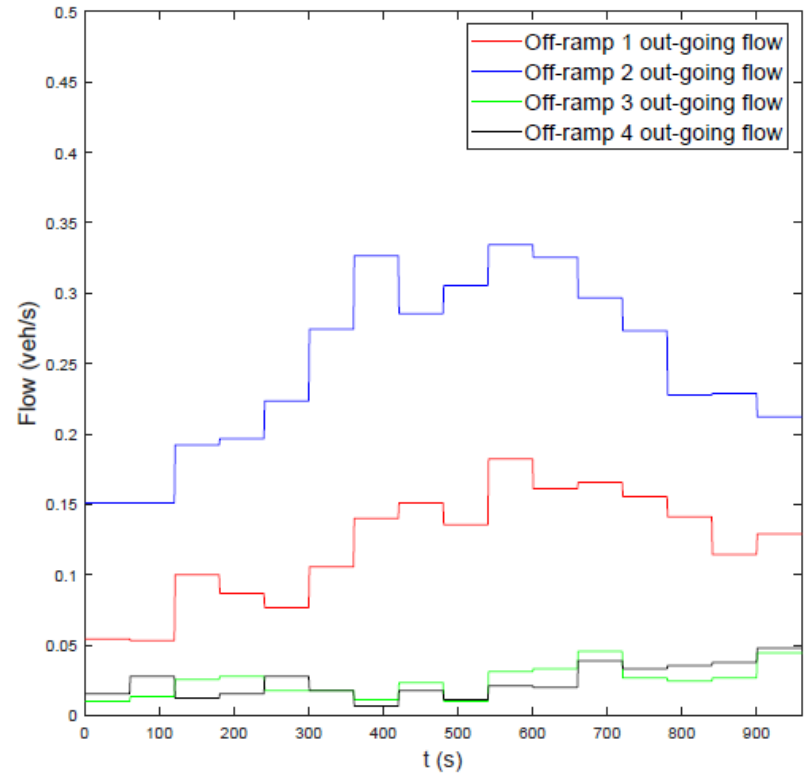
Model Predictive Control (MPC)



Demand flows and out-going flows

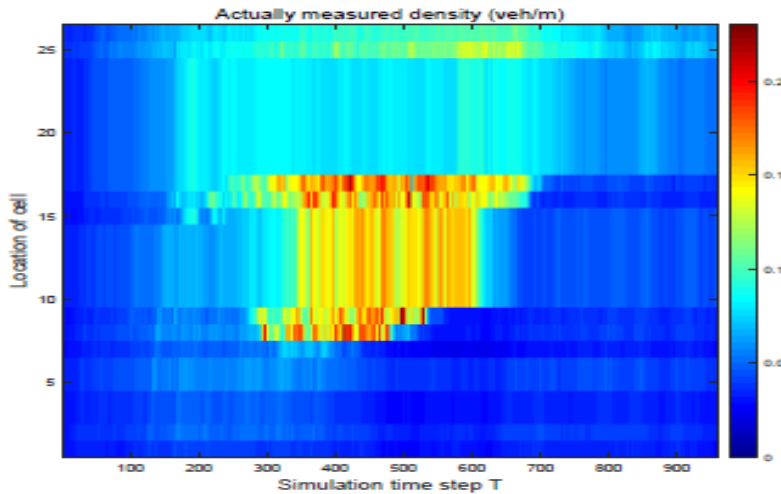


(a) Mainline demand and on-ramp demands

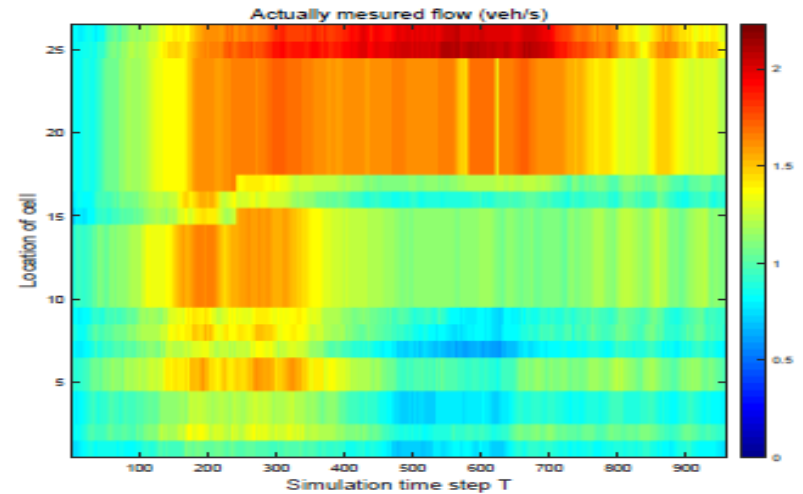


(b) Out-going flow

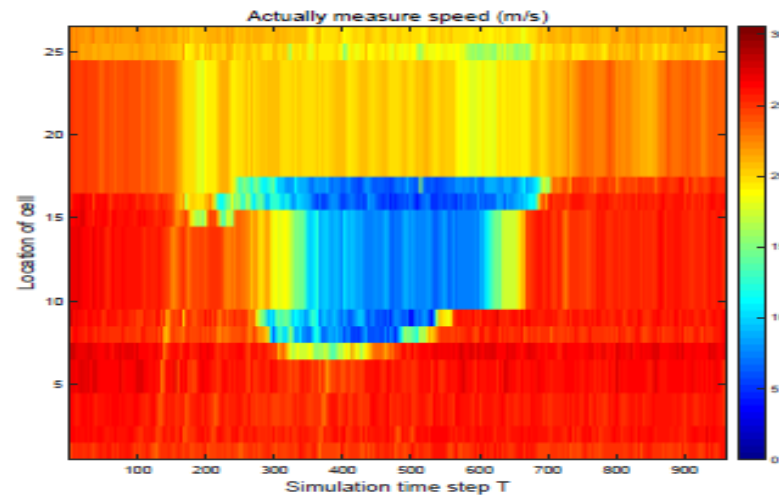
Density, speed and flow from actual measured data 11/06/2018



(a) Density

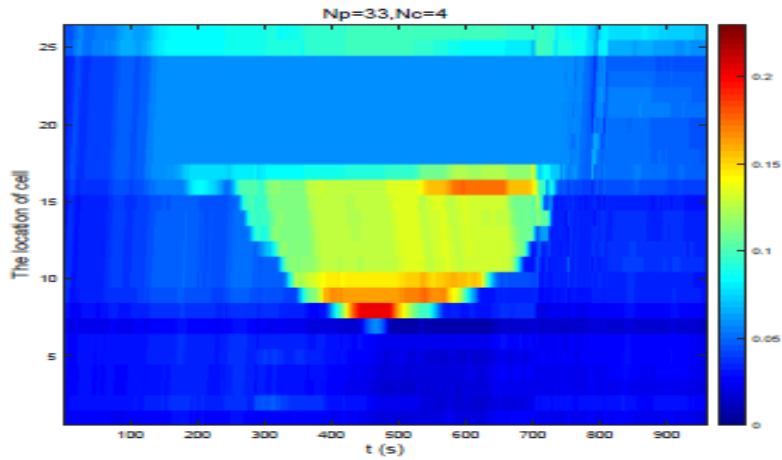


(b) Flow

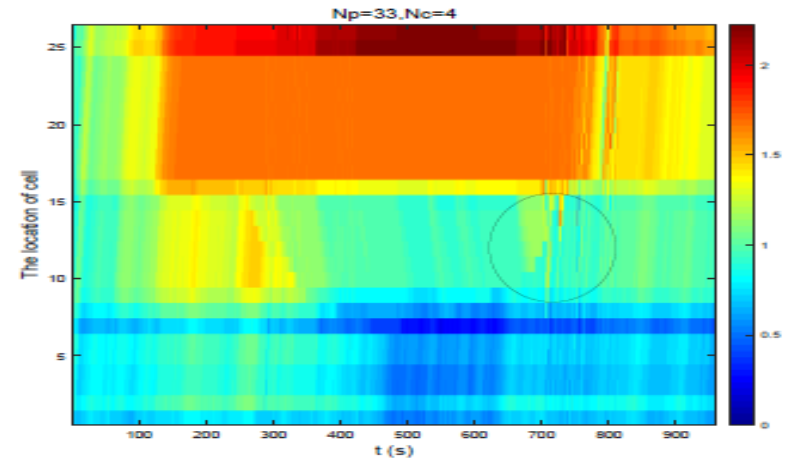


(c) Speed

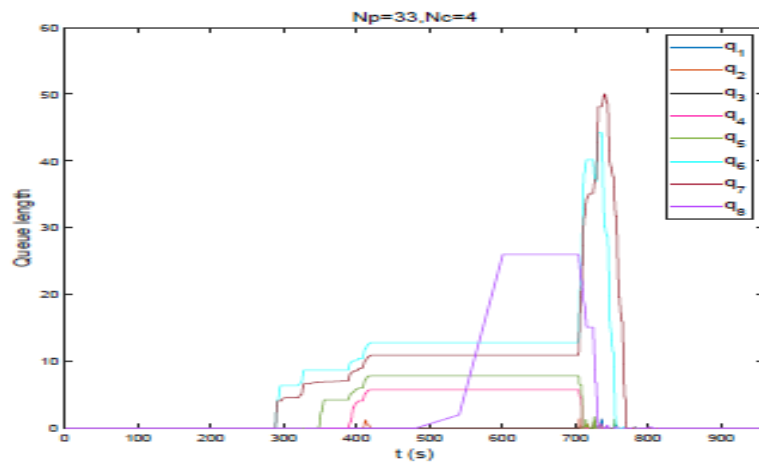
Numerical results for inequality relaxation for $N_p=33, N_c=4$



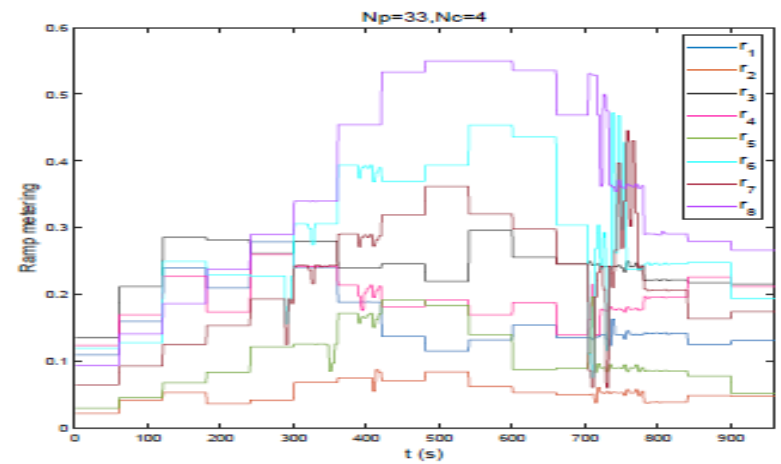
(a) Density



(b) Flow

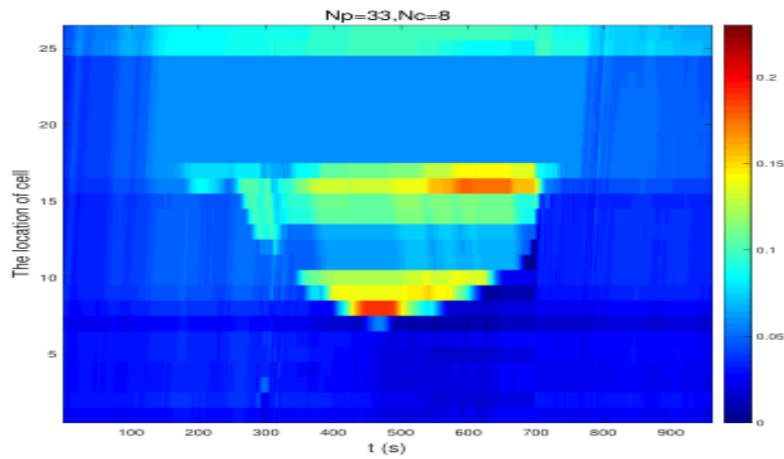


(c) Queue length

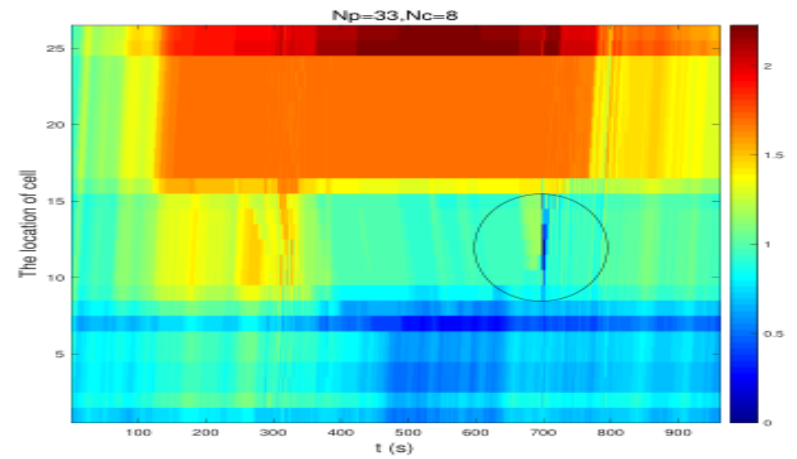


(d) Ramp metering

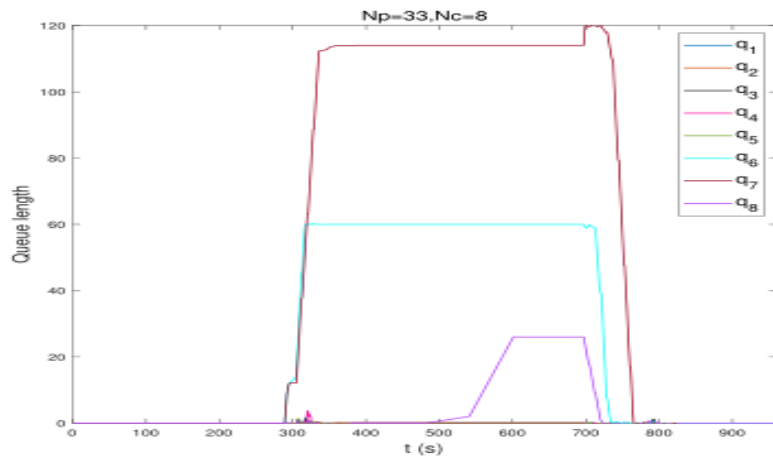
Numerical results for inequality relaxation for $N_p=33, N_c=8$



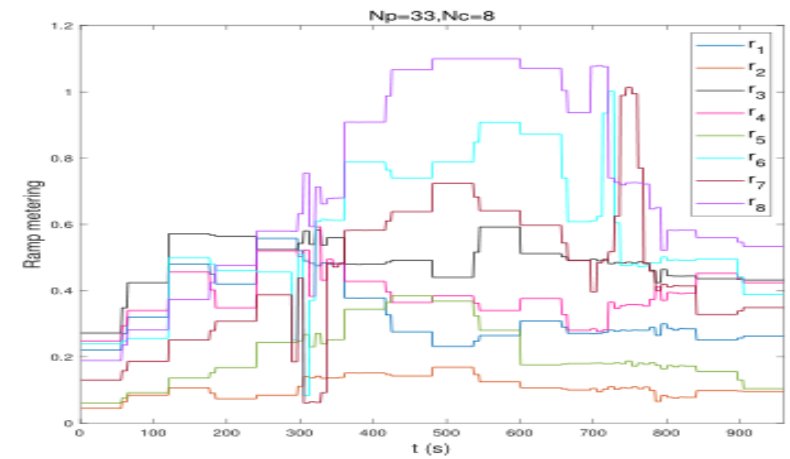
(a) Density



(b) Flow

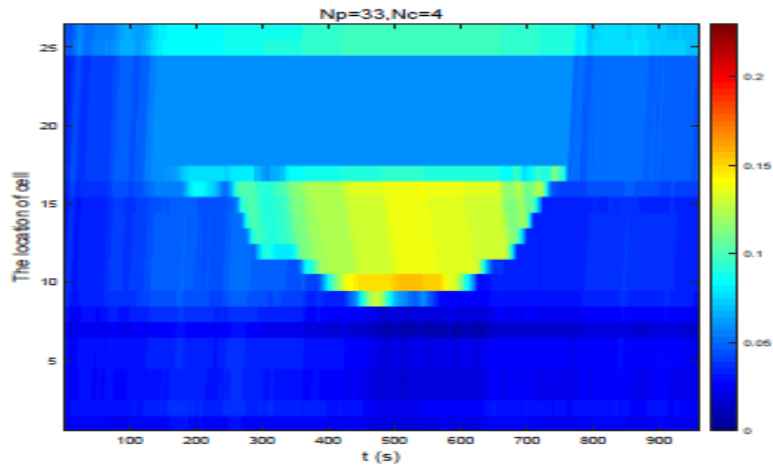


(c) Queue length

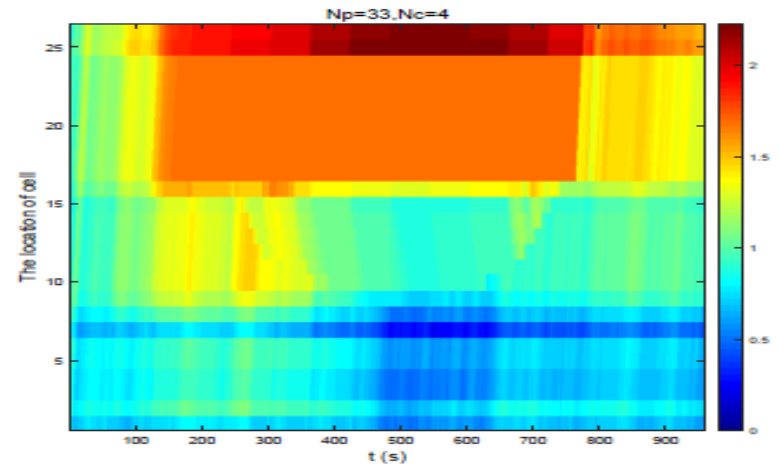


(d) Ramp metering

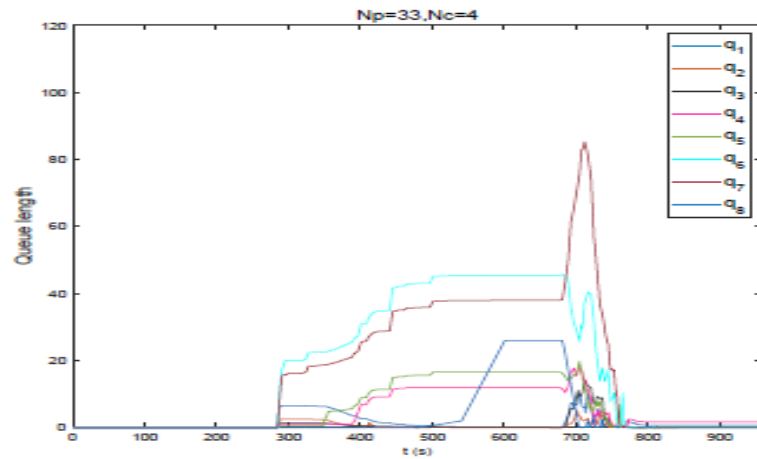
Numerical results for approximated model for $N_p=33, N_c=4$



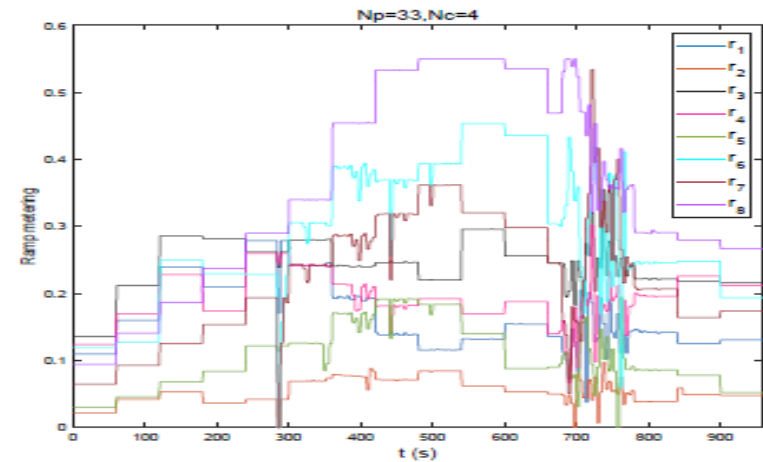
(a) Density



(b) Flow

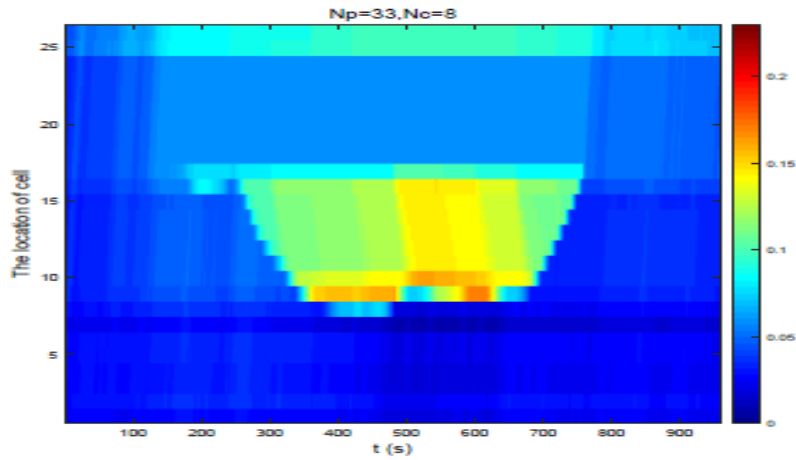


(c) Queue length

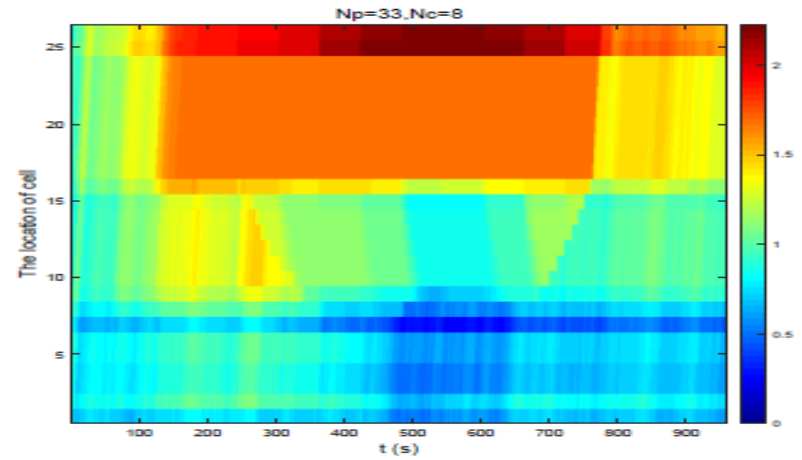


(d) Ramp metering

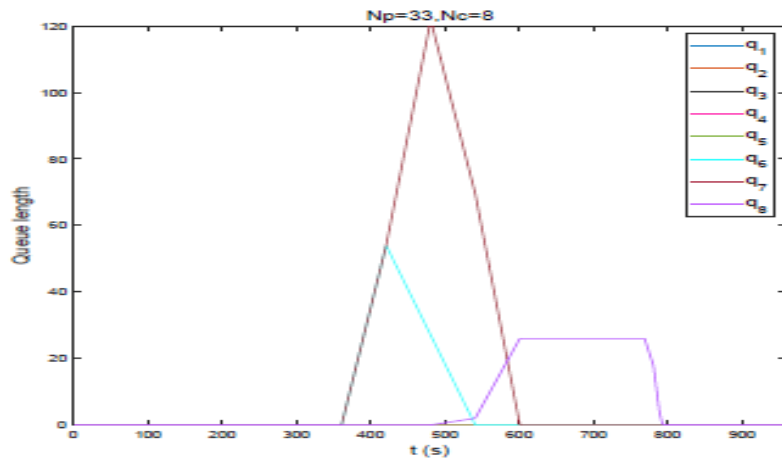
Numerical results for approximated model for $N_p=33, N_c=8$



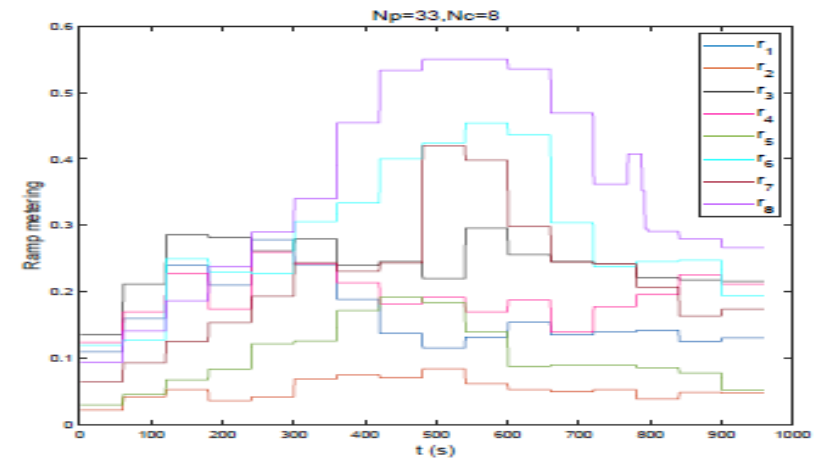
(a) Density



(b) Flow



(c) Queue length



(d) Ramp metering

Comparison of total delay and ramp delay

Name of model	Total delay	Ramp delay
No control	1270.3164h	0h
Inequality relaxation model with $N_c = 4$	781.3429h	87.3461h
Inequality relaxation model with $N_c = 8$	772.7768h	325.4634h
Approximated model with $N_c = 4$	765.9473h	199.2427h
Approximated model with $N_c = 8$	761.2329h	105.0768h

ACKNOWLEDGEMENTS

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THANK YOU

For Your Attention

