

On Simulation and Optimization of Freeway Network Operations

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Progress from last PSG meeting

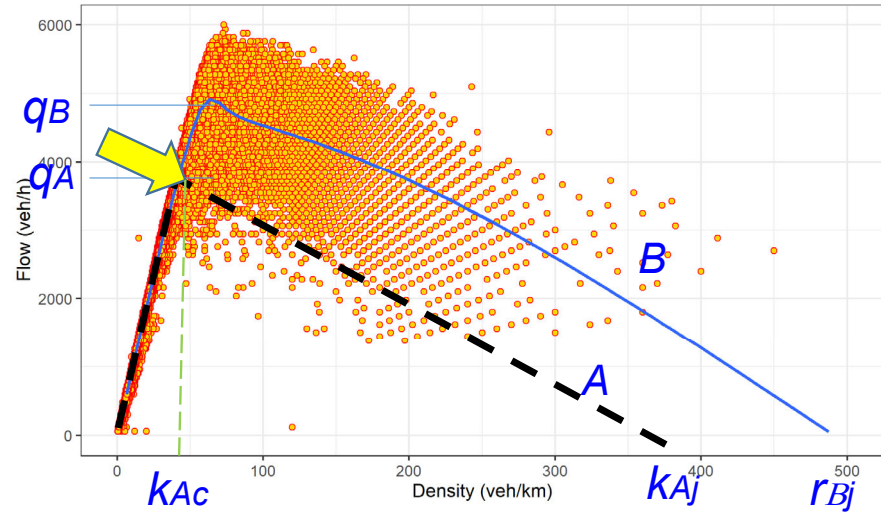
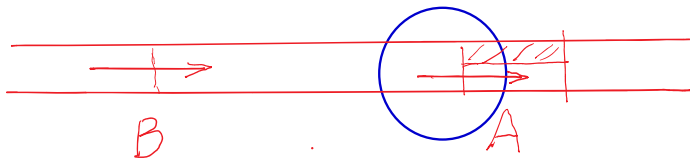
Analysis and computation of optimal VSL and RM under non-recurrent events with lane closure

1. Analysis of the Problem

Assume that 1 or 2 lanes close at location A
due to the occurrence of a non-recurrent event



Capacity drop due to Lane Closure



Fundamental diagram of traffic flow

- Lane closure leads to capacity drop as shown above
- Without control, traffic will build up quickly at location A, so $k_A \uparrow$ → congestion
- by reducing speed limits of vehicles toward A, it is possible to max the throughput at A

2. The optimisation model

$$\max_{r_p(t), V_l(t)} \sum_{t=t_0}^{T_{end}} q_{critical}\{k_i[r_p(t), V_l(t)]\} \Delta t$$

subject to

$$\bullet \frac{n_i \partial k_i}{\partial t} + \frac{\partial}{\partial x} \Delta q_i(t) = 0, \quad 0 \leq k_i(t) \leq k_{jam,i}$$

$$\Delta q_i(t) = q_i^{in}(t-1) - q_i^{out}(t-1) + r_i(t-1) - s_i(t-1)$$

$$r_i(t) = \min \left[\begin{array}{l} n_{i,r} q_{c,r}; \beta_{i,r} v_{f,r} k_{i,r}(t-1); \\ \max \{ r_i^\alpha(t); queue_{i,r}(t) \} \end{array} \right]$$

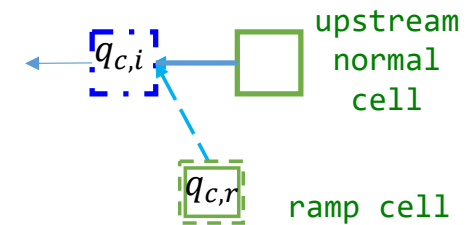
- $queue_{i,r}(t) = arr_{i,r}(t-1) - \frac{1}{T} (\hat{m}_{i,r} - m_{i,r}(t-1));$
- $m_{i,r}(t) = m_{i,r}(t-1) + T(arr_{i,r}(t-1) - r_i(t)) \leq \hat{m}_{i,r}$
- $r_i^\alpha(t) = r_i^\alpha(t-1) + \alpha_i(t) \frac{L_i}{T} (n_i k_{c,i} - k_i(t-1)) > 0$

↑

Metering parameter

- Capacity constraints

$$\sum_{t=\tau}^{t_{end}} T \left\{ \sum_{i=1}^{rs} (\Delta q_i(t) - q_{c,i}) \right\} \leq 0$$



$$\left(\begin{array}{l} \text{Green} \\ \text{duration} \end{array} \quad g_i(t) = \frac{r_i(t)}{r_s(t)} C_i \right)$$

$$T = t_{record}/3600$$

➤ As in practice, $V_i(t)$, $\alpha_i(t)$ and $\beta_{i,r}(t)$ cannot be changed continuously, they are approximated by

$$V_i(t) = \begin{cases} V_{i1} & \text{if } t \in [t_0, t_0 + T_c) \\ V_{i2} & \text{if } t \in [t_0 + T_c, t_0 + 2T_c) \\ \vdots & \\ V_{im} & \text{if } t \in [t_0 + (m-1)T_c, t_0 + mT_c) \end{cases} \quad (i=1, \dots, n_v)$$

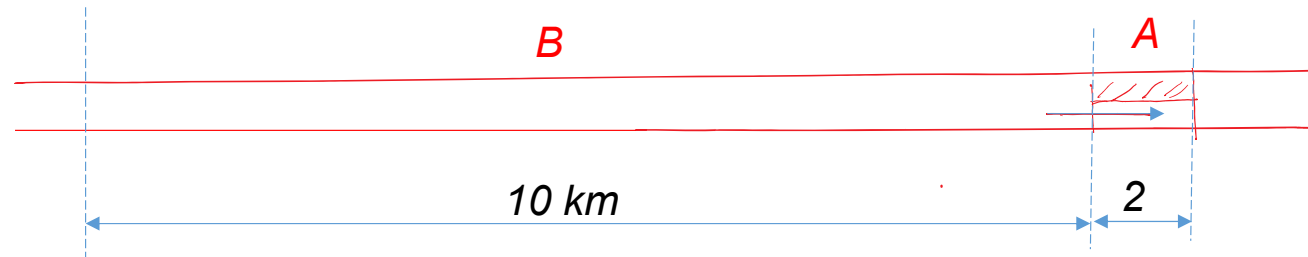
$$\alpha_i(t) = \begin{cases} \alpha_{i1} & \text{if } t \in [t_0, t_0 + T_c) \\ \alpha_{i2} & \text{if } t \in [t_0 + T_c, t_0 + 2T_c) \\ \vdots & \\ \alpha_{im} & \text{if } t \in [t_0 + (m-1)T_c, t_0 + mT_c) \end{cases} \quad (i=1, \dots, n_r)$$

➤ Hence, the optimal control problem becomes the following optimization problem:

$$\max_{V_{lj}, \alpha_{pj}, \beta_{pj}} \sum_{t=t_0}^{t_0+mT_c} q_{critical} \{k_i(t, V_{lj}, \alpha_{pj}, \beta_{pj})\}$$

subject to the dynamic equations and capacity constraints as given before

3 A simple test Example



$v_f = 100 \text{ km/h}$,

$k_{jB} = 0.1 \text{ (vehicle /m)}$

$$q(k) = v_f k \left(1 - \frac{k}{k_j} \right)$$

Capacity drops by 50% at A

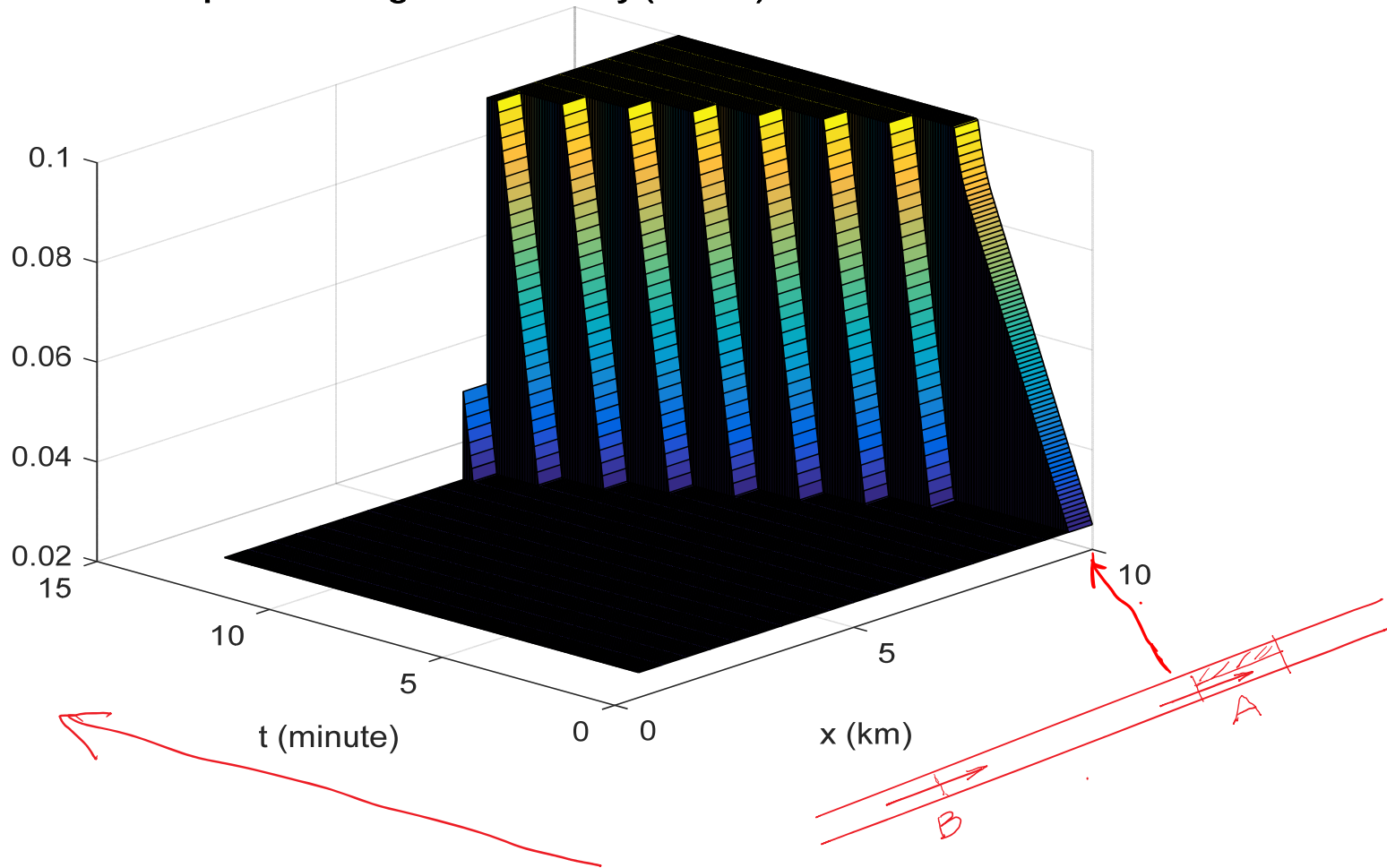
Duration for handling the incident $T = 12 \text{ minutes}$

The **Objective** is to avoid traffic jam & maximize the throughput over the time period T

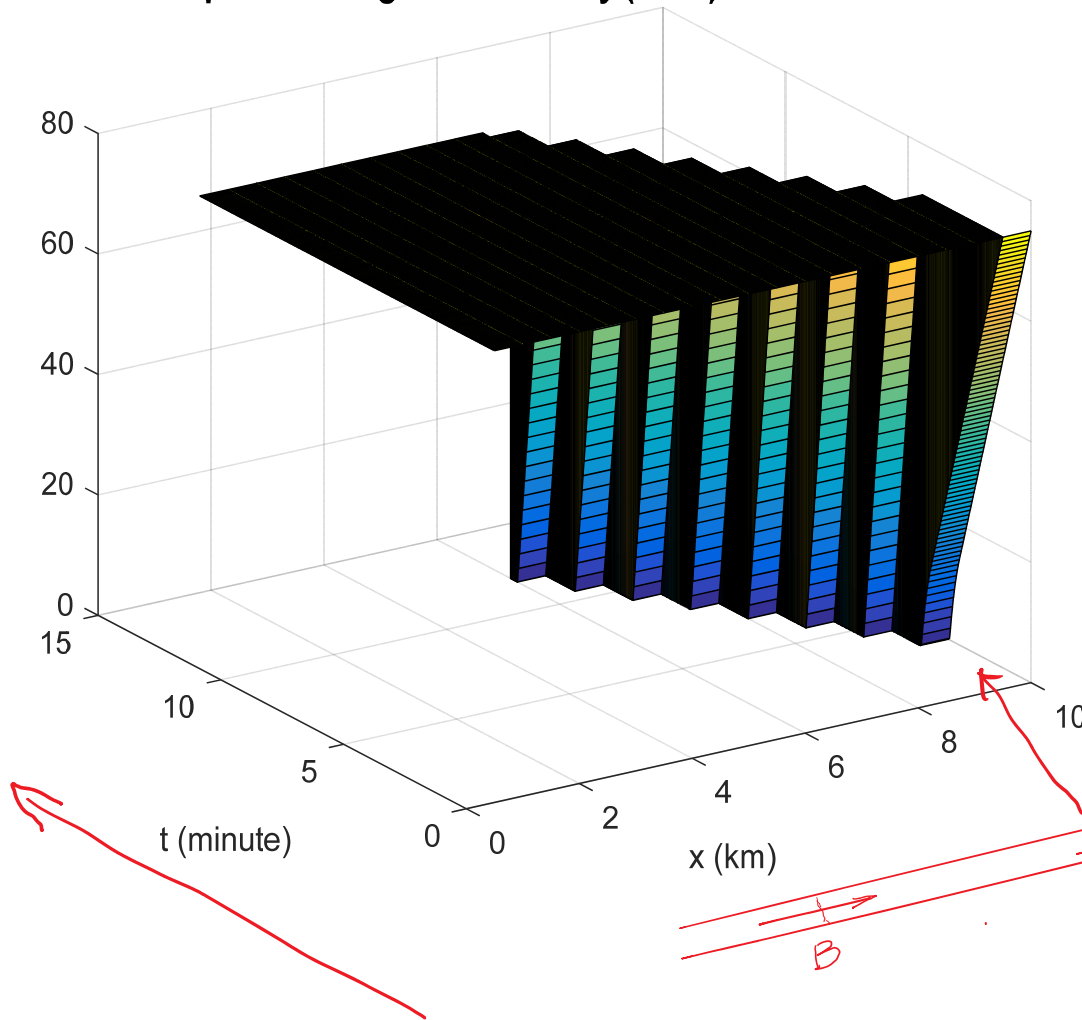
The Cell Transmission model is used for numerical simulation of the traffic flow

Results with no Control

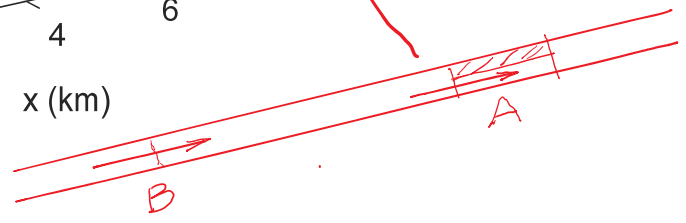
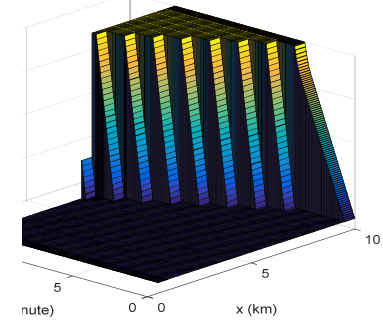
Surface plot showing traffic density (Veh/m) as a function of t and x



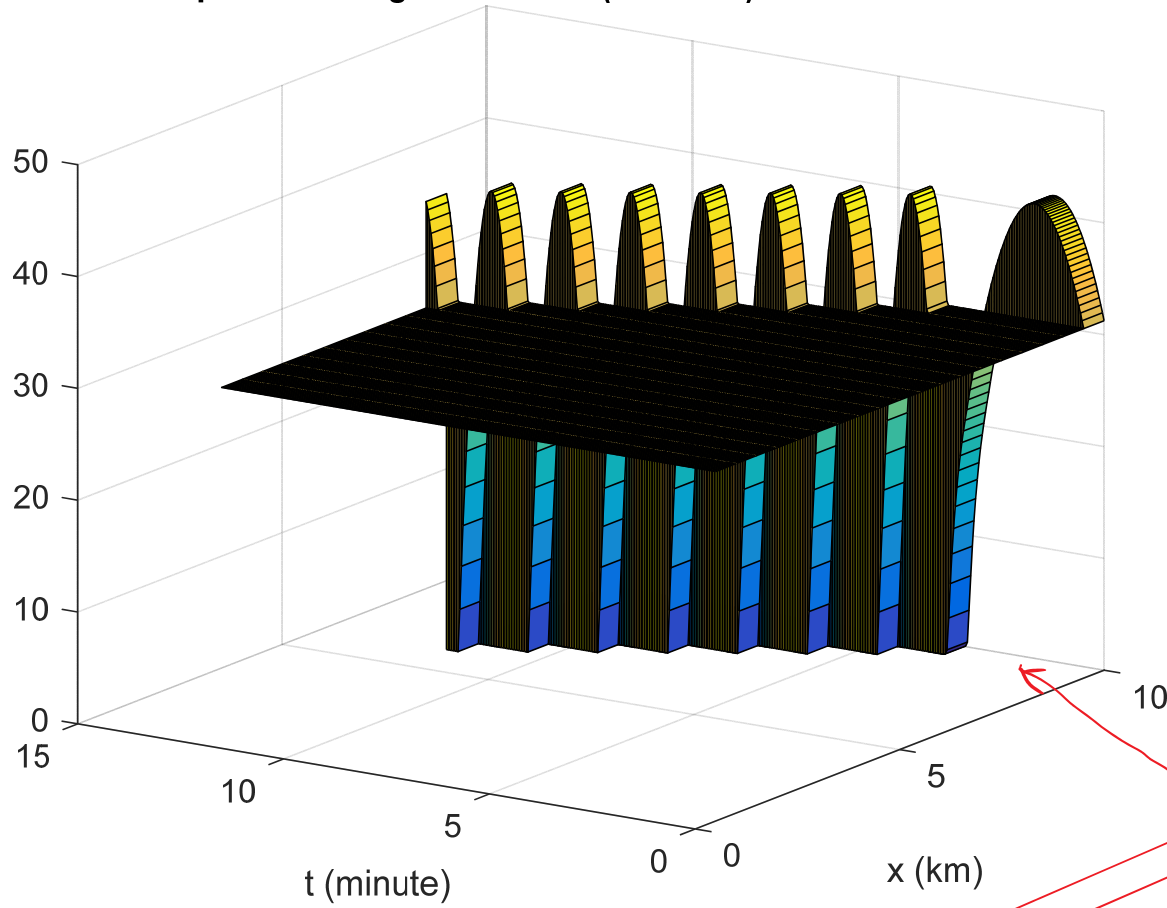
Surface plot showing traffic velocity (km/h) as a function of t and x



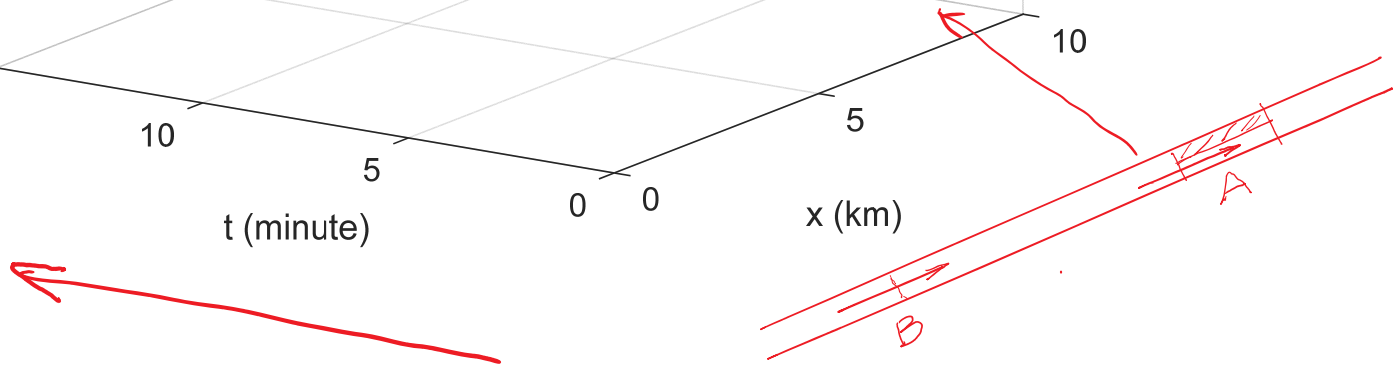
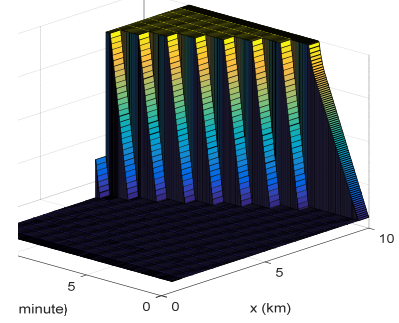
Following traffic density (Veh/m) as a function of t and x



Surface plot showing traffic flow (Veh/min) as a function of t and x

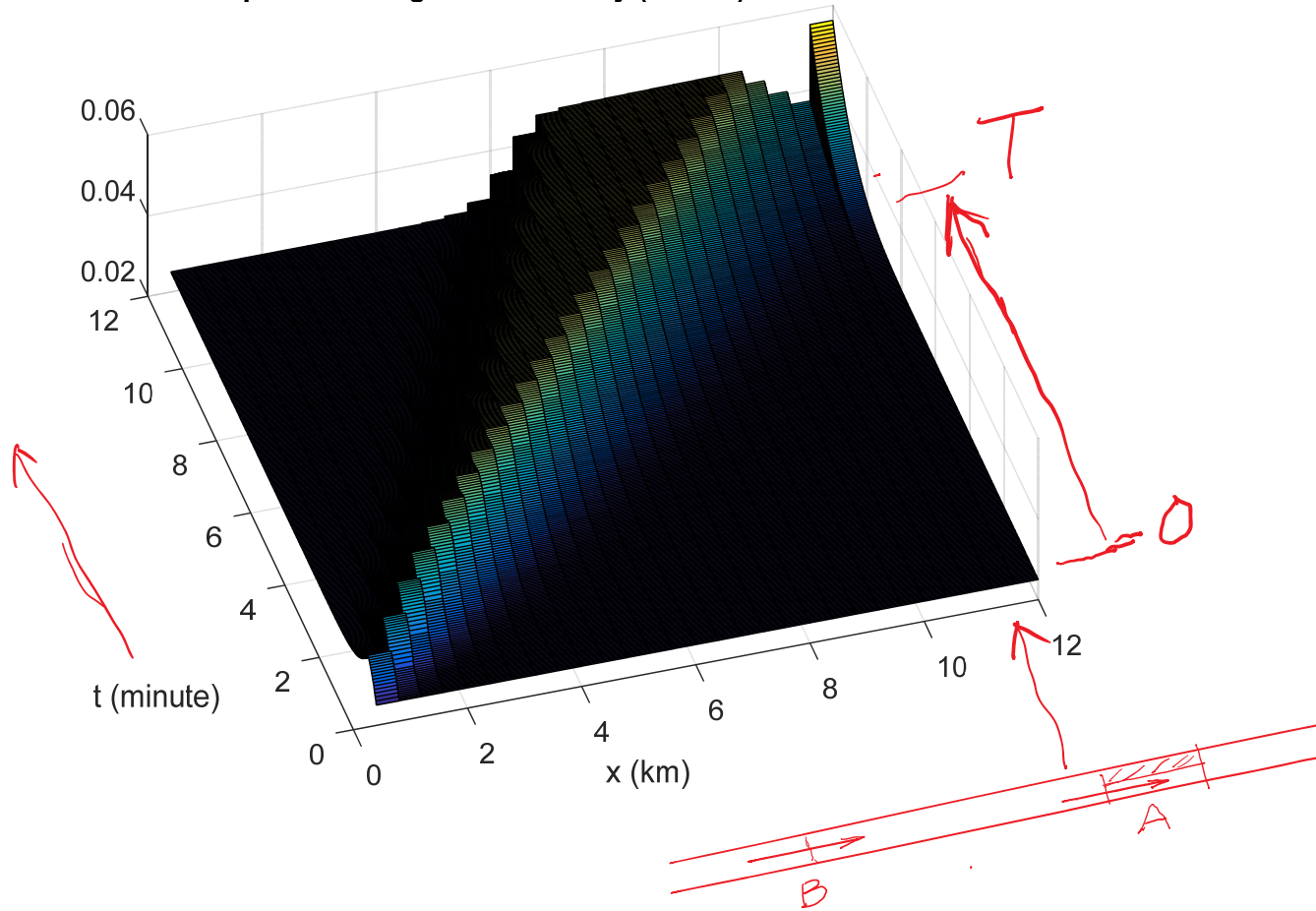


Showing traffic density (Veh/m) as a function of t and x

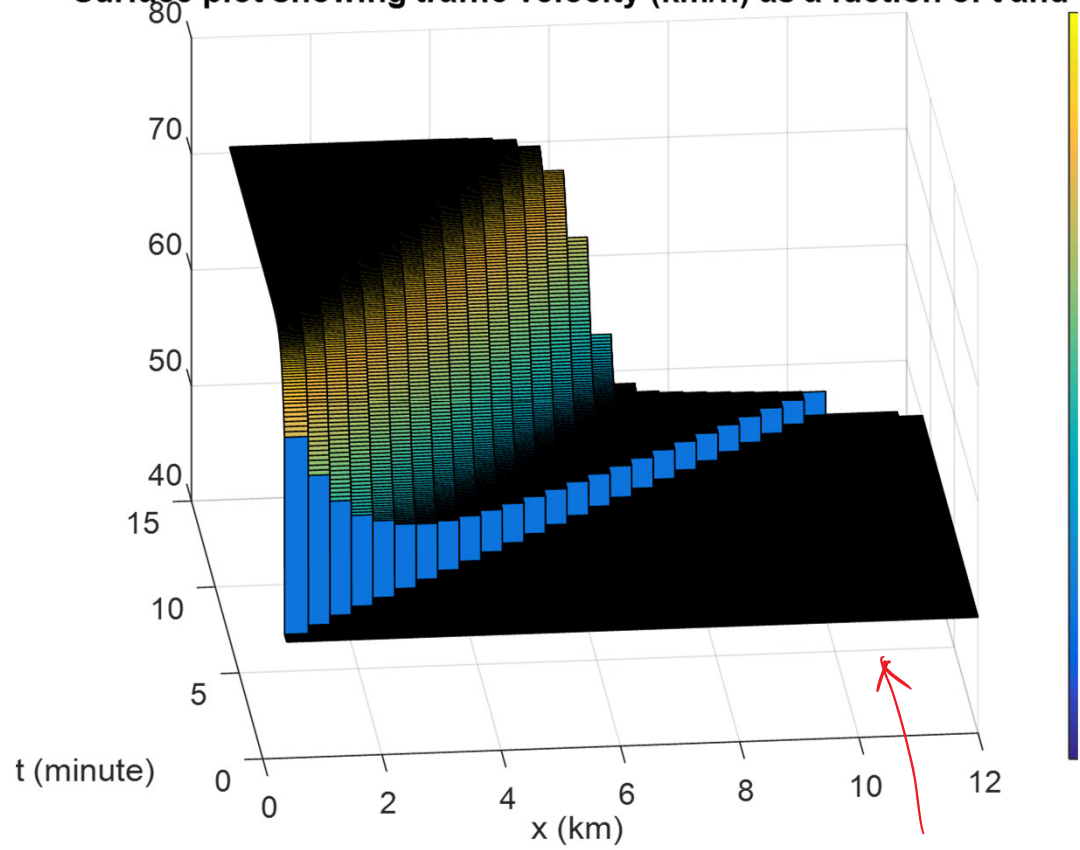


Results with VSL Control

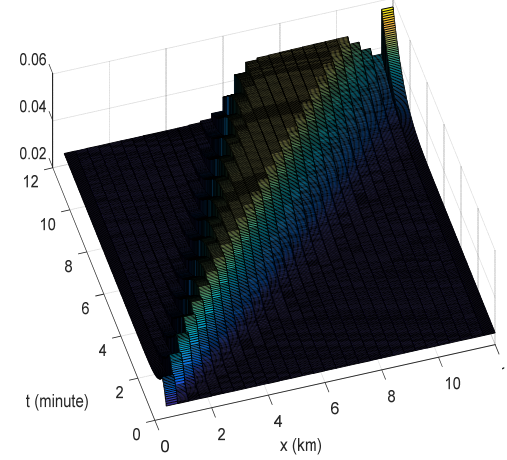
Surface plot showing traffic density (Veh/m) as a function of t and x



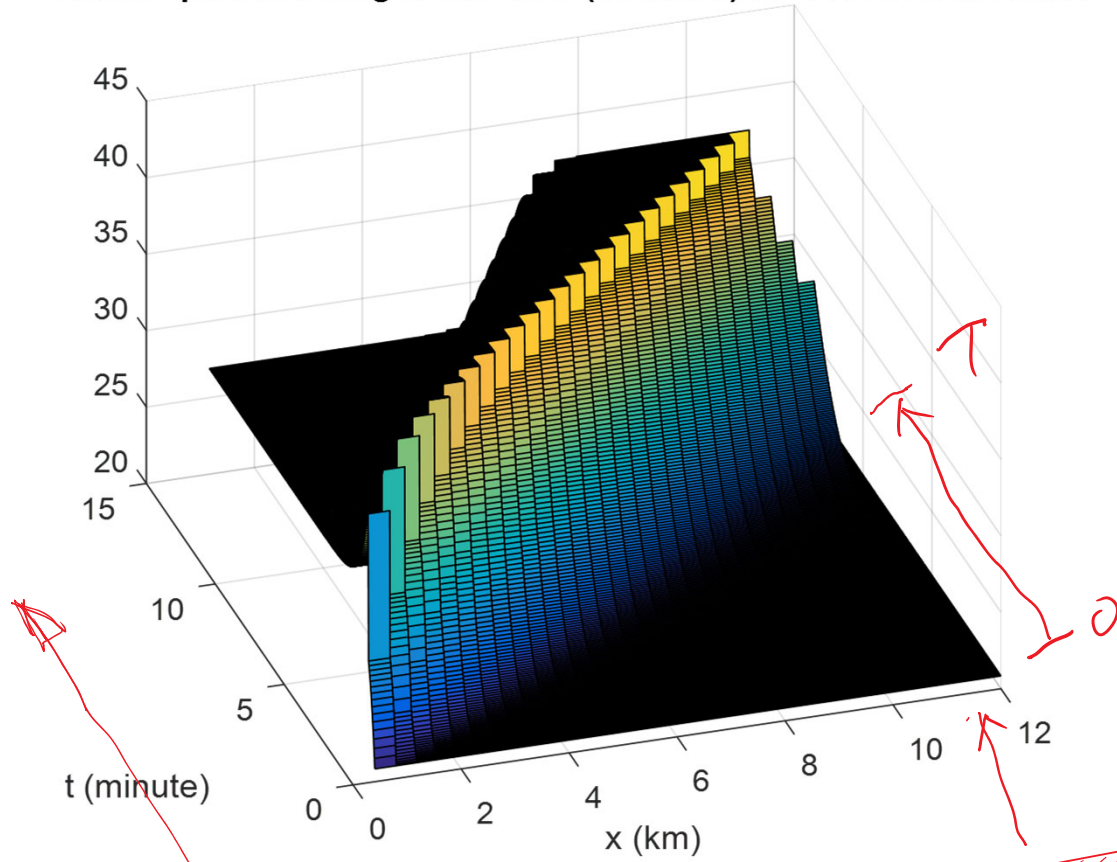
Surface plot showing traffic velocity (km/h) as a function of t and x



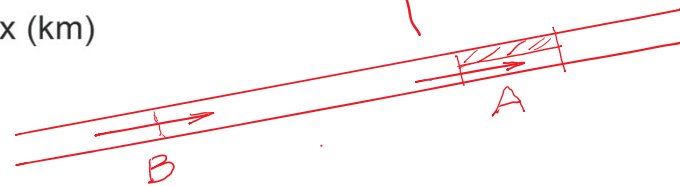
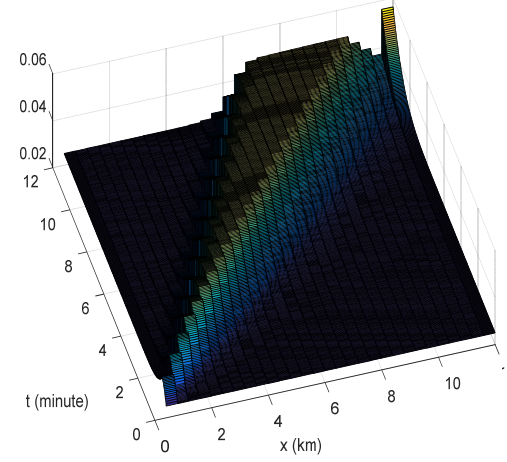
Surface plot showing traffic density (Veh/m) as a function of t and x



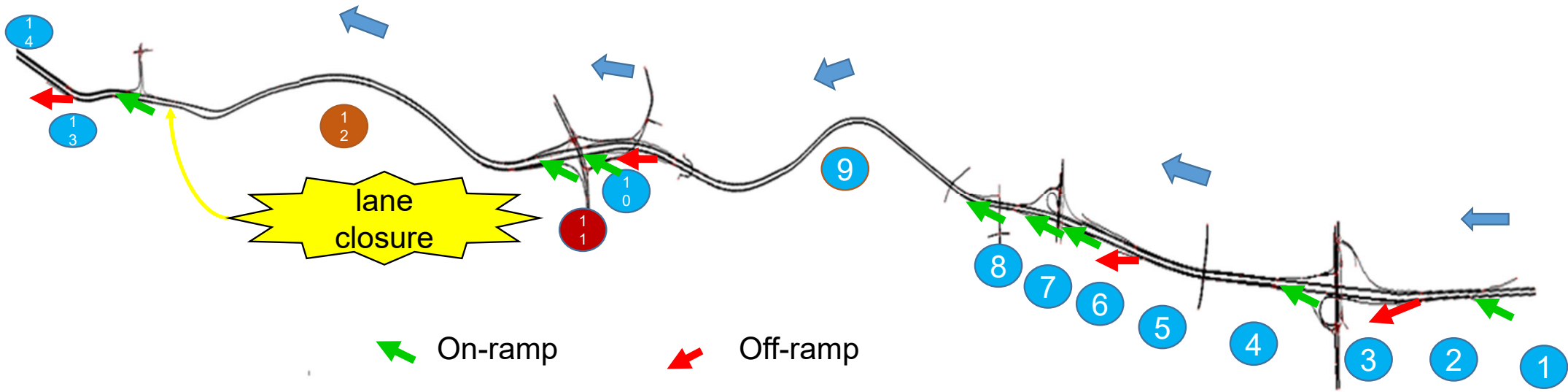
Surface plot showing traffic flow (Veh/min) as a function of t and x



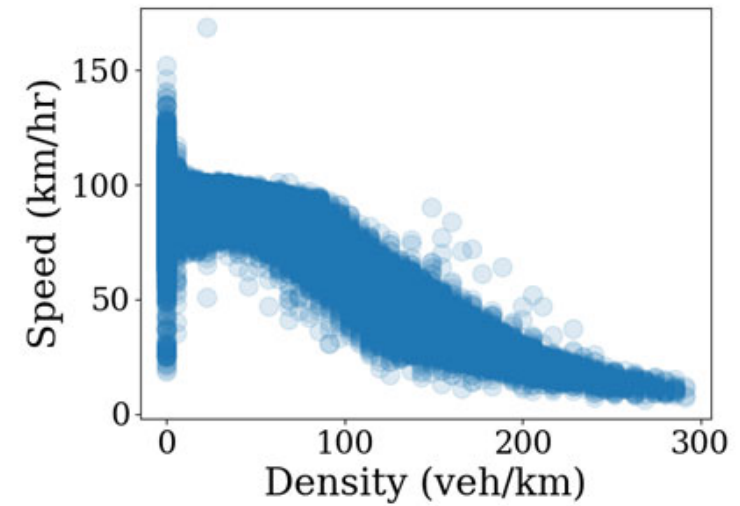
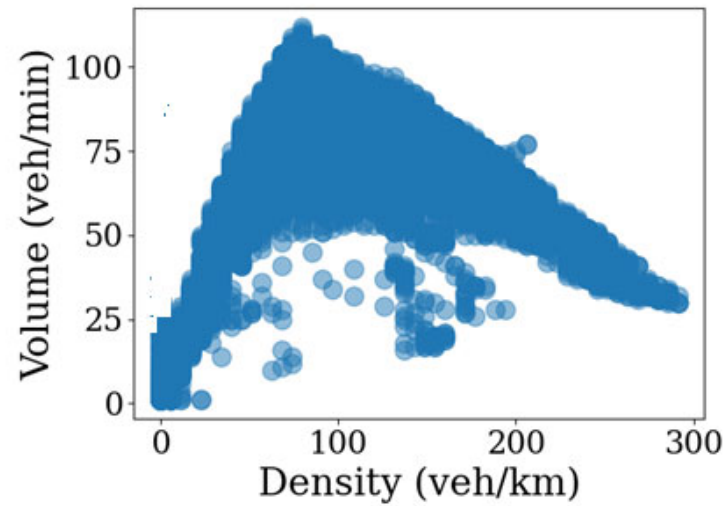
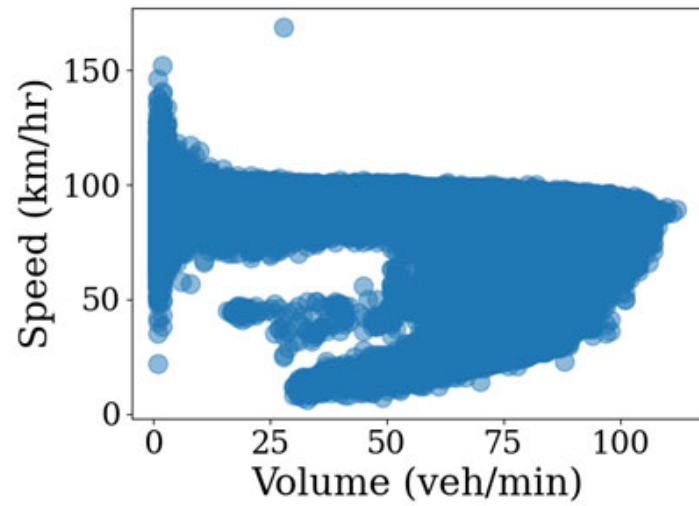
Surface plot showing traffic density (Veh/m) as a function of t and x



4. Study on the Kwinana Freeway Northbound



Fundamental diagrams





	1	2	3	4	5	6	7	8	9	10	11	12
VSLs	LUMS 212	LUMS 211	LUMS 210	LUMS 209	LUMS 208	LUMS 207	LUMS 206	LUMS 205	LUMS 204	LUMS 203	LUMS 202	LUMS 201
Road seg.	18 (1443)	19 (269)	20 (757)	21-24 (577)	25 (218)	26 (493)	27 (665)	28 (550)	29 (746)	30 (711)	31 (274)	32 (219)
Lanes	3	3	3	4	4	3	3	3	3	3	4	5

Scenario 1: 1 or 2 lanes close due to a non-recurrent event at the location shown above

- Numerical results will be presented in the next meeting

ACKNOWLEDGEMENTS

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THANK YOU
For Your Attention

